





Report III - Master Plan Class EA Report

Appendix A-1

Water Servicing Analysis







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Appendix A-1 (01)

Growth Projections

Projections

Population

PRESSURE_DISTRICT	2001	2011	2021	2031
1	136,591.41	152,419.29	168,389.93	180,021.80
2	60,545.13	66,026.39	71,013.93	72,735.76
3	14,112.76	14,531.66	14,557.99	14,546.53
4	35,178.74	37,274.95	41,326.78	41,705.79
5	72,636.84	73,320.43	78,062.70	80,893.09
6	82,533.00	91,780.13	110,909.32	113,650.15
7	7,423.23	9,271.73	15,743.66	109,151.71
9	19.05	25.86	41.52	117.01
10	339.25	460.49	739.38	2,083.62
11	14,456.99	15,797.15	16,347.82	16,325.45
12	5,312.80	5,598.53	6,008.66	6,084.97
13	678.80	721.23	853.89	813.59
14	428.50	455.29	539.03	
16	13,611.04	14,313.00	20,368.40	29,642.60
17	2,238.36	2,319.23	2,568.17	2,472.08
18	19,513.75	22,676.24	29,338.98	29,377.67
19	398.46	411.69	411.03	411.03
20	269.46	271.15	271.15	271.15
21	113.71	133.99	140.10	139.65
22	4,908.98	6,120.57	11,636.75	18,863.15
Grand Total	471,310.25	513,929.00	589,269.20	719,820.40

Jobs

PRESSURE_DISTRICT	2001	2011	2021	2031
1	99,301.95	106,527.43	115,082.20	126,022.96
2	35,017.02	42,223.11	50,068.27	58,645.11
3	1,108.65	1,089.27	1,066.40	1,126.66
4	4,688.05	4,919.90	5,168.49	
5	17,641.82	18,836.50	20,813.39	
6	19,395.33	22,314.84	29,476.08	
7	5,104.22	6,298.11	7,887.92	8,953.72
10	61.57	64.15	62.47	66.66
11	5,301.53	5,956.73	6,337.61	7,015.21
12	597.84	622.77	634.21	687.67
13	59.90	61.93	72.07	81.29
16	5,009.73	5,906.08	7,209.78	8,048.44
17	498.76	506.85	574.07	645.56
18	4,824.60	6,012.81	7,777.35	11,653.11
19	87.89	86.38	84.12	89.74
20	33.54	33.15	32.54	34.69
22	300.50	823.00	1,531.00	2,382.00
Grand Total	201,033.90	224,294.00	255,899.00	302,118.00







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Demand Projections

Max Day Demands

PRESSURE_DISTRICT	2001 (m3/d)	2011 (m3/d)	2021 (m3/d)	2031 (m3/d)
1	133,591.86	146,845.84	160,876.70	173,545.02
2	54,535.93	61,571.85	68,643.86	74,136.91
3	9,044.16	9,285.41	9,289.32	9,313.78
4	23,545.03	24,923.32	27,483.68	28,103.23
5	52,755.85	53,787.24	57,660.58	60,485.18
6	59,605.37	66,671.80	81,873.16	91,970.79
7	7,108.14	8,838.06	13,547.92	70,146.96
9	11.43	15.52	24.91	70.20
10	235.56	309.65	476.11	1,284.83
11	11,430.99	12,575.79	13,104.25	13,443.18
12	3,498.56	3,682.96	3,934.98	4,008.57
13	438.43	464.94	549.81	530.43
14	257.10	273.17	323.42	308.15
16	10,771.68	11,658.96	15,970.13	21,970.75
17	1,602.37	0.55	1,839.42	1,818.94
18	14,217.04	0.00	21,647.61	23,686.22
19	284.78	291.93	290.36	293.28
20	179.11	179.93	179.61	180.73
21	68.22	80.40	84.06	83.79
22	3,101.65	4,100.30	7,778.17	12,556.53
Grand Total	386,283.26	405,557.61	485,578.08	587,937.48

Peak Hour Demands

PRESSURE_DISTRICT	2001 (m3/d)	2011 (m3/d)	2021 (m3/d)	2031 (m3/d)
1	200,387.79	220,268.76	241,315.06	260,317.53
2	81,803.89	92,357.77	102,965.79	111,205.37
3	13,566.23	13,928.12	13,933.99	13,970.68
4	35,317.55	37,384.98	41,225.53	42,154.84
5	79,133.77	80,680.86	86,490.88	90,727.77
6	89,408.06	100,007.69	122,809.73	137,956.18
7	10,662.20	13,257.08	20,321.87	105,220.44
9	17.15	23.27	37.37	105.31
10	353.35	464.47	714.17	1,927.25
11	17,146.49	18,863.68	19,656.38	20,164.77
12	5,247.84	5,524.44	5,902.48	6,012.86
13	657.64	697.41	824.72	795.64
14	385.65	409.76	485.13	462.23
16	16,157.52	17,488.44	23,955.19	32,956.13
17	2,403.56	0.83	2,759.12	2,728.41
18	21,325.56	0.00	32,471.42	35,529.32
19	427.17	437.90	435.54	439.93
20	268.67	269.89	269.42	271.09
21	102.34	120.59	126.09	125.68
22	4,652.47	6,150.46	11,667.25	18,834.79
Grand Total	579,424.88	608,336.41	728,367.12	881,906.22





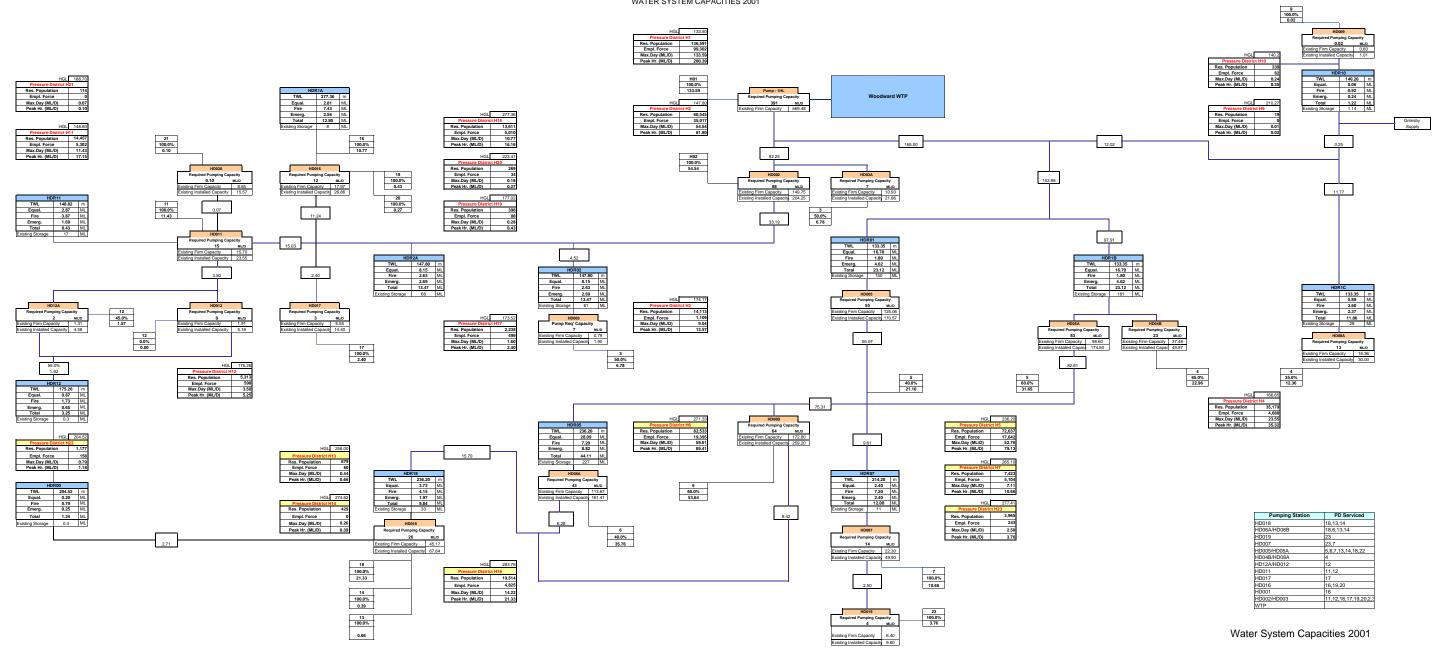


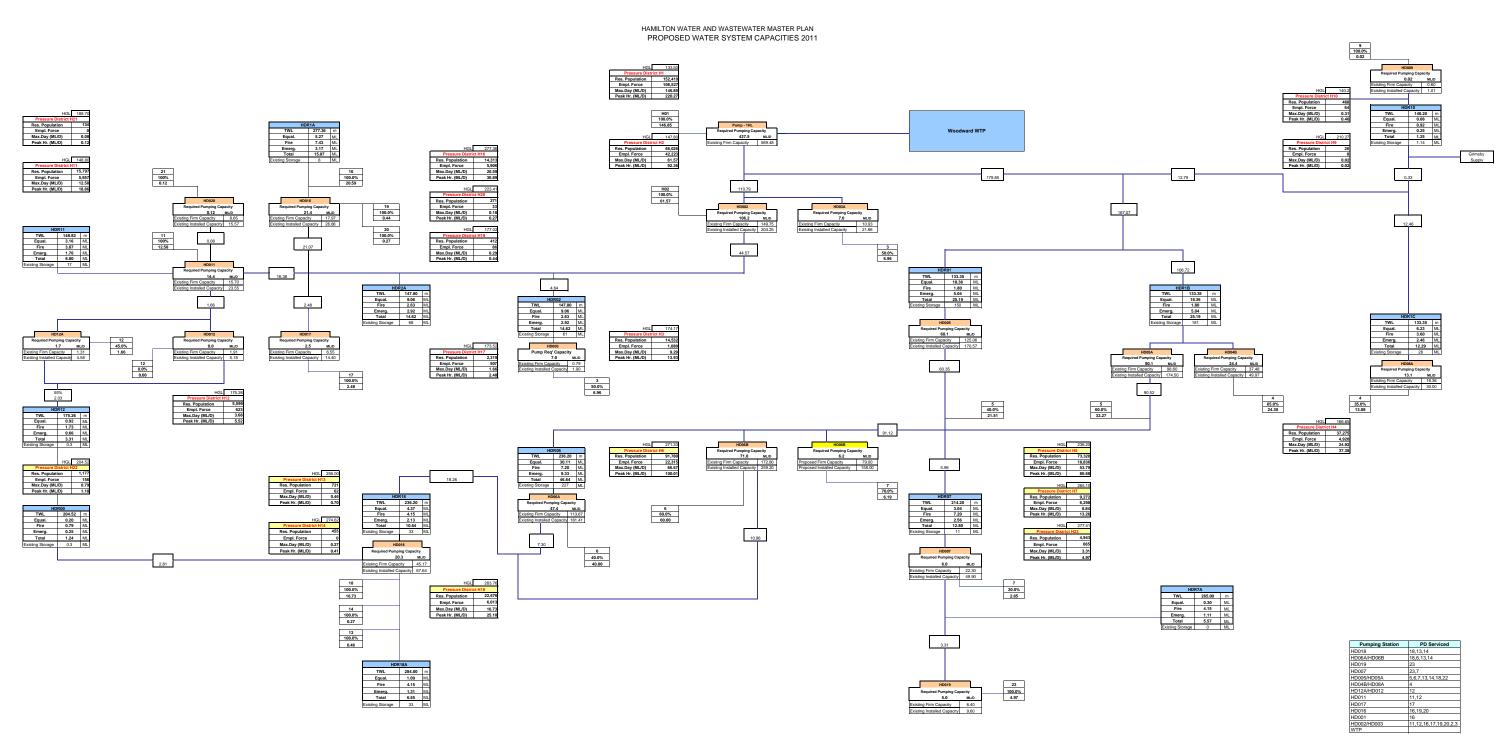
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Appendix A-1 (03)

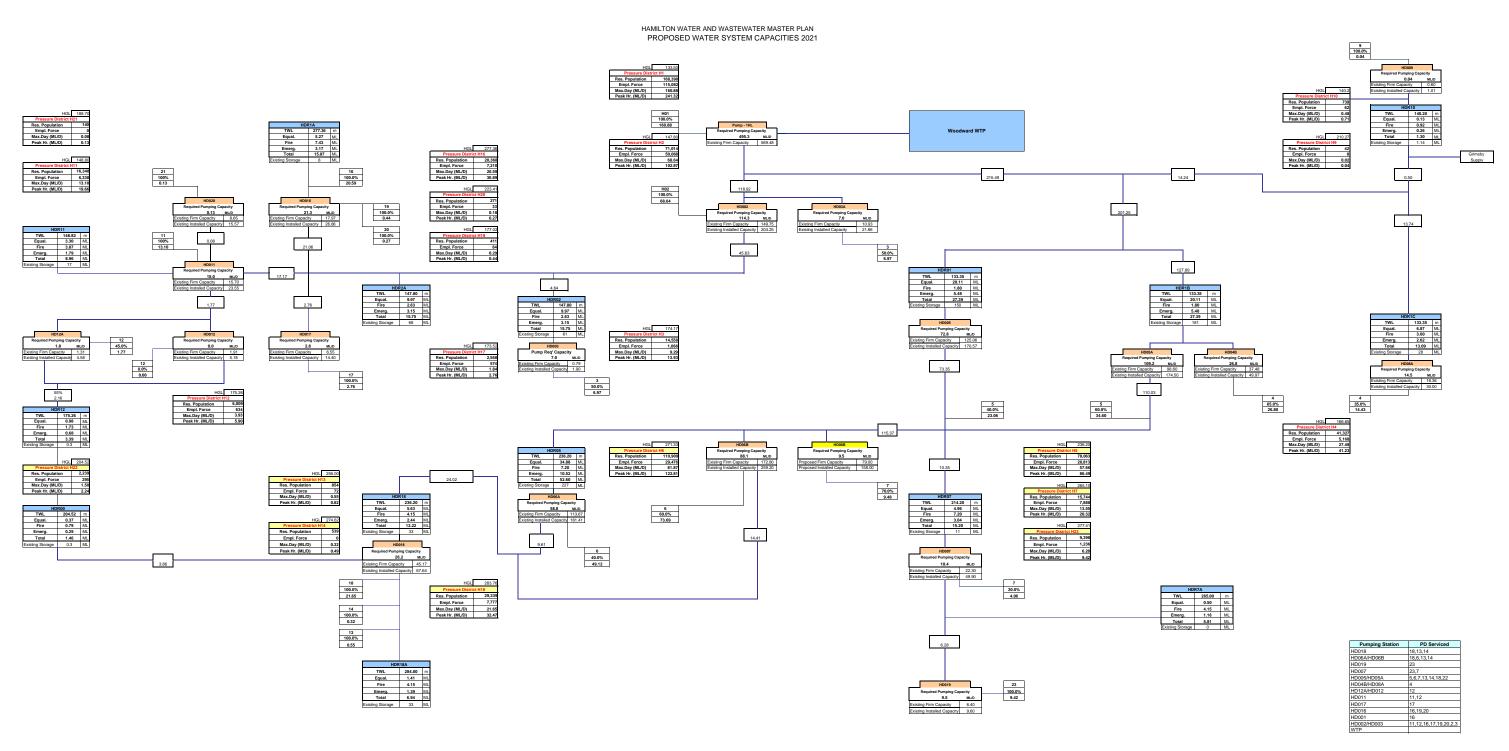
Water System Capacities

HAMILTON WATER AND WASTEWATER MASTER PLAN WATER SYSTEM CAPACITIES 2001



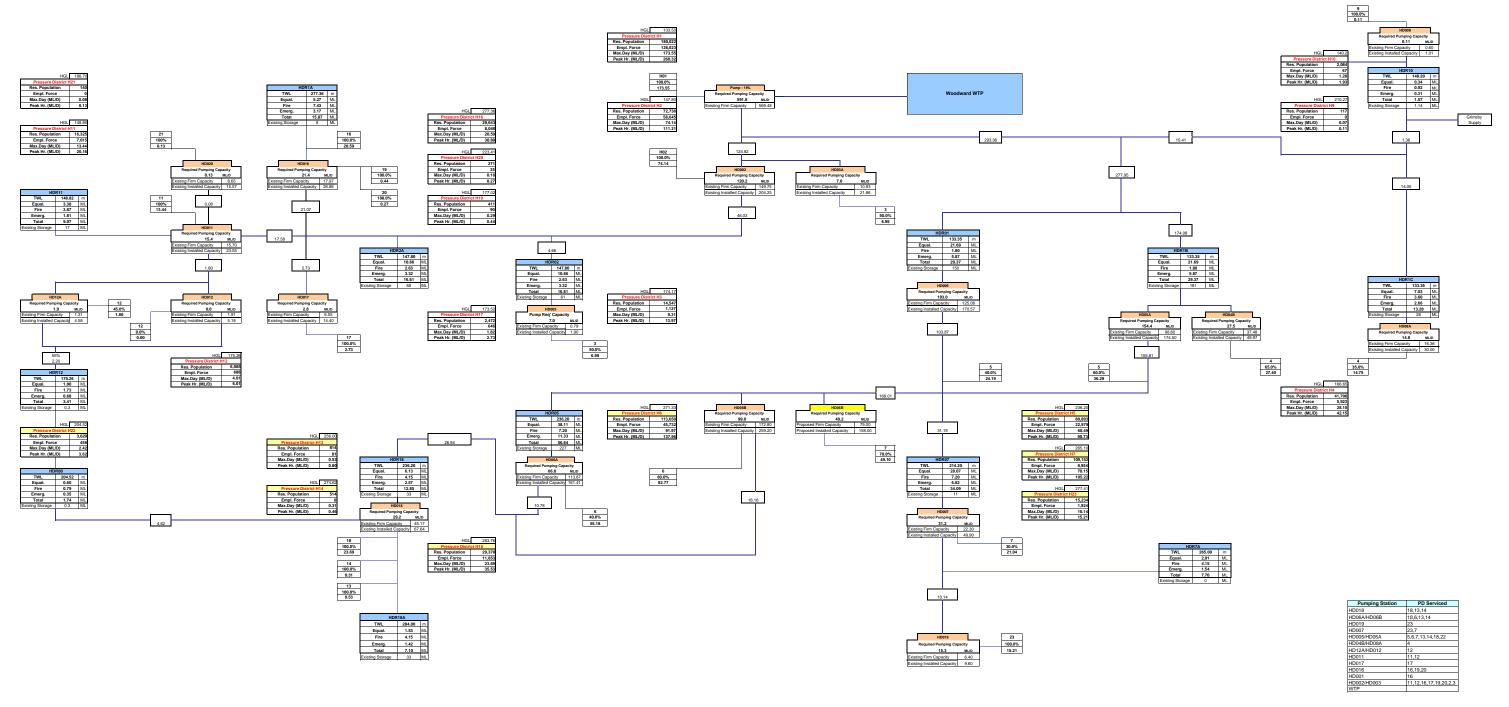


Water System Capacities 2011



Water System Capacities 2021

HAMILTON WATER AND WASTEWATER MASTER PLAN PROPOSED WATER SYSTEM CAPACITIES 2031









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Appendix A-1 (04)

Ducasuma Diatriat	Description		Design Flo	ws (MLD)	
Pressure District	Description	2001	2011	2021	2031
22	Supply Requirement				
	HD-22 Max	0.78	0.79	1.50	2.42
	Pumping Capacity				
	% HD018	1.45	1.26	1.78	2.79
	Surplus/Deficit Capacity	0.67	0.47	0.29	0.37
13	Supply Requirement				
	HD-13 Peak	0.66	0.70	0.82	0.80
	Pumping Capacity				
	% HD018	1.23	1.12	0.98	0.92
	Surplus/Deficit Capacity	0.57	0.42	0.16	0.12
14	Supply Requirement				
	HD-14 Peak	0.39	0.41	0.49	0.46
	Pumping Capacity				
	% HD018	0.72	0.66	0.58	0.53
	Surplus/Deficit Capacity	0.33	0.25	0.09	0.07
18	Supply Requirement				
	HD-22 Max	0.78	0.79	1.50	2.42
	HD-14 Peak	0.39	0.41	0.49	0.46
	HD-13 Peak	0.66	0.70	0.82	0.80
	HD-18 Peak	22.40	26.30	35.09	35.53
	Total Requirement	24.22	28.20	37.90	39.21
	Pumping Capacity				
	HD018 Firm Capacity	45.17	45.17	45.17	45.17
	Surplus/Deficit Capacity	20.95	16.97	7.27	5.96
6	Supply Requirement				
	HD-14 Max	0.26	0.27	0.32	0.31
	HD-13 Max	0.44	0.46	0.55	0.53
	HD-18 Max	14.93	17.53	23.39	23.69
	HD-06 Max	58.89	65.87	80.13	91.97
	Total Requirement	74.52	84.14	104.39	116.50
	Pumping Capacity				
	HD06A Firm Capacity	113.67	113.67	113.67	113.67
	HD06B Firm Capacity	172.80	172.80	172.80	172.80
	Total Capacity	286.47	286.47	286.47	286.47
00	Surplus/Deficit Capacity	211.95	202.33	182.08	169.97
23	Supply Requirement	105	4.07	2.42	45.01
	HD-23 Peak	4.65	4.97	9.42	15.21
	Pumping Capacity	0.77	0.00	7.00	0.00
	% HD007 Firm Capacity	6.77	6.08	7.06	2.82
7	Surplus/Deficit Capacity	2.12	1.11	-2.36	-12.39
7	Supply Requirement	40.00	10.00	20.00	105.00
	HD-07 Peak	10.66	13.26	20.32	105.22
	HD-23 Peak	4.65	4.97	9.42	15.21
	Total Requirement	15.31	18.22	29.74	120.43
	Pumping Capacity	22.22	00.00	60.00	20.00
	HD007 Firm Capacity	22.30	22.30	22.30	22.30
	Surplus/Deficit Capacity	6.99	4.08	-7.44	-98.13

Pressure District	Description		Design Flows (MLD)			
Pressure District	Description	2001	2011	2021	2031	
5	Supply Requirement					
	HD-14 Max	0.26	0.27	0.32	0.31	
	HD-13 Max	0.44	0.46	0.55	0.53	
	HD-18 Max	14.93	17.53	23.39	23.69	
	HD-06 Max	58.89	65.87	80.13	91.97	
	HD-23 Max	3.10	3.31	6.28	10.14	
	HD-07 Max	7.11	8.84	13.55	70.15	
	HD-05 Max	52.76	53.79	57.66	60.49	
	Total Requirement	137.48	150.08	181.88	257.27	
	Pumping Capacity					
	HD005 Firm Capacity	125.06	125.06	125.06	125.06	
	HD05A Firm Capacity	98.60	98.60	98.60	98.60	
	Total Capacity	223.66	223.66	223.66	223.66	
	Surplus/Deficit Capacity	86.17	73.58	41.77	-33.61	
4	Supply Requirement					
	HD-04 Peak	35.32	37.38	41.23	42.15	
	Pumping Capacity					
	HD04B Firm Capacity	37.48	37.48	37.48	37.48	
	HD08A Firm Capacity	16.36	16.36	16.36	16.36	
	Total Capacity	53.84	53.84	53.84	53.84	
	Surplus/Deficit Capacity	18.52	16.46	12.61	11.69	
9	Supply Requirement					
	HD-09 Peak	0.02	0.02	0.04	0.11	
	Total Requirement	0.02	0.02	0.04	0.11	
	Pumping Capacity					
	HD009 Firm Capacity	0.60	0.60	0.60	0.60	
	Surplus/Deficit Capacity	0.58	0.58	0.56	0.49	
19	Supply Requirement					
	HD-19 Peak	0.43	0.44	0.44	0.44	
	Pumping Capacity					
	% HD016 Firm Capacity	0.67	0.64	0.47	0.37	
	Surplus/Deficit Capacity	0.24	0.20	0.03	-0.07	
20	Supply Requirement		İ			
	HD-20 Peak	0.27	0.27	0.27	0.27	
	Pumping Capacity		İ			
	% HD016 Firm Capacity	0.42	0.39	0.29	0.23	
	Surplus/Deficit Capacity	0.15	0.12	0.02	-0.04	

5 5			Design Flo	ws (MLD)	
Pressure District	Description	2001	2011	2021	2031
16	Supply Requirement				
	HD-19 Peak	0.43	0.44	0.44	0.44
	HD-20 Peak	0.27	0.27	0.27	0.27
	HD-16 Max	10.77	11.66	15.97	20.59
	Total Requirement	11.47	12.37	16.68	21.31
	Pumping Capacity				
	HD016 Firm Capacity	17.97	17.97	17.97	17.97
	Surplus/Deficit Capacity	6.50	5.60	1.29	-3.34
12	Supply Requirement				
	HD-12 Max	3.50	3.68	3.93	4.01
	Pumping Capacity				
	HD012 Firm Capacity	1.91	1.91	1.91	1.91
	HD12A Firm Capacity	1.31	1.31	1.31	1.31
	Total Requirement	3.22	3.22	3.22	3.22
	Surplus/Deficit Capacity	-0.28	-0.46	-0.71	-0.79
21	Supply Requirement				
	HD-21 Peak	0.10	0.12	0.13	0.13
	Pumping Capacity				
	HD020 Firm Capacity	8.65	8.65	8.65	8.65
	Surplus/Deficit Capacity	8.55	8.53	8.52	8.53
11	Supply Requirement				
	HD-11 Max	11.43	12.58	13.10	13.44
	HD-12 Max	1.57	1.66	1.77	1.80
	HD-21 Peak	0.10	0.12	0.13	0.13
	Total Requirement	13.11	14.35	15.00	15.37
	Pumping Capacity				
	HD011 Firm Capacity	15.70	15.70	15.70	15.70
	Surplus/Deficit Capacity	2.59	1.34	0.70	0.33
17	Supply Requirement				
	HD-17 Peak	2.40	2.48	2.76	2.73
	Pumping Capacity				
	HD017 Firm Capacity	6.55	6.55	6.55	6.55
	Surplus/Deficit Capacity	4.15	4.07	3.79	3.82
3	Supply Requirement				
	HD-03 Peak	13.57	13.93	13.93	13.97
	Pumping Capacity				
	HD003 Firm Capacity	0.79	0.79	0.79	0.79
	HD002-PD3 Firm Capacity	149.75	149.75	149.75	149.75
	Total Capacity	150.54	150.54	150.54	150.54
_	Surplus/Deficit Capacity	136.97	136.61	136.61	136.57
2	Supply Requirement	ļ			
	HD-19 Max	0.28	0.29	0.29	0.29
	HD-20 Max	0.18	0.18	0.18	0.18
	HD-16 Max	10.77	11.66	15.97	20.59
	HD-12 Max	3.50	3.68	3.93	4.01
	HD-21 Peak	0.10	0.12	0.13	0.13
	HD-11 Max	11.43	12.58	13.10	13.44
	HD-17 Peak	2.40	2.48	2.76	2.73
	HD-03 Max	9.04	9.29	9.29	9.31
	HD-02 Max	54.54	61.57	68.64	74.14
	Total Requirement	92.25	101.85	114.30	124.82
	Pumping Capacity	440.75	440.75	440.75	440.75
	HD002 Firm Capacity	149.75	149.75	149.75	149.75
	Surplus/Deficit Capacity	57.50	47.90	35.45	24.93

Pressure District	Description		Design Flo	ws (MLD)	
Pressure District	Description	2001	2011	2021	2031
1	Supply Requirement				
	HD-19 Max	0.28	0.29	0.29	0.29
	HD-20 Max	0.18	0.18	0.18	0.18
	HD-16 Max	10.77	11.66	15.97	20.59
	HD-12 Max	3.50	3.68	3.93	4.01
	HD-21 Peak	0.10	0.12	0.13	0.13
	HD-11 Max	11.43	12.58	13.10	13.44
	HD-17 Peak	2.40	2.48	2.76	2.73
	HD-03 Max	9.04	9.29	9.29	9.31
	HD-02 Max	54.54	61.57	68.64	74.14
	HD-14 Max	0.26	0.27	0.32	0.31
	HD-13 Max	0.44	0.46	0.55	0.53
	HD-18 Max	14.93	17.53	23.39	23.69
	HD-06 Max	58.89	65.87	80.13	91.97
	HD-23 Max	3.10	3.31	6.28	10.14
	HD-07 Max	7.11	8.84	13.55	70.15
	HD-05 Max	52.76	53.79	57.66	60.49
	HD-04 Max	23.55	24.92	27.48	28.10
	HD-10 Max	0.24	0.31	0.48	1.28
	HD-09 Max	0.01	0.02	0.02	0.07
	HD-01 Max	133.59	146.85	160.88	173.55
	Total Requirement	387.12	424.02	485.04	585.10
	Pumping Capacity				
	Woodward Firm Capacity	569.48	569.48	569.48	569.48
	Surplus/Deficit Capacity	182.36	145.46	84.44	-15.62





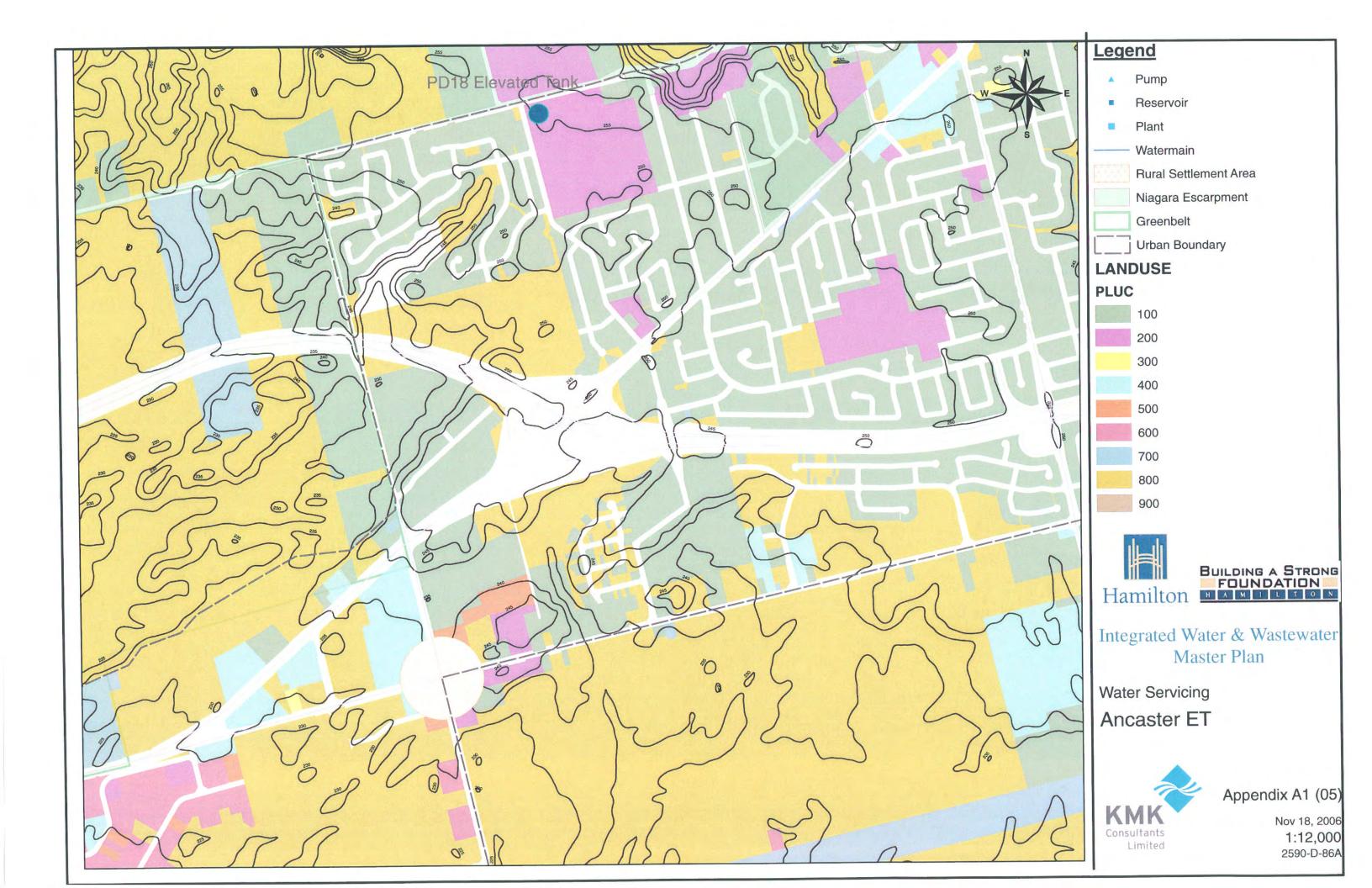


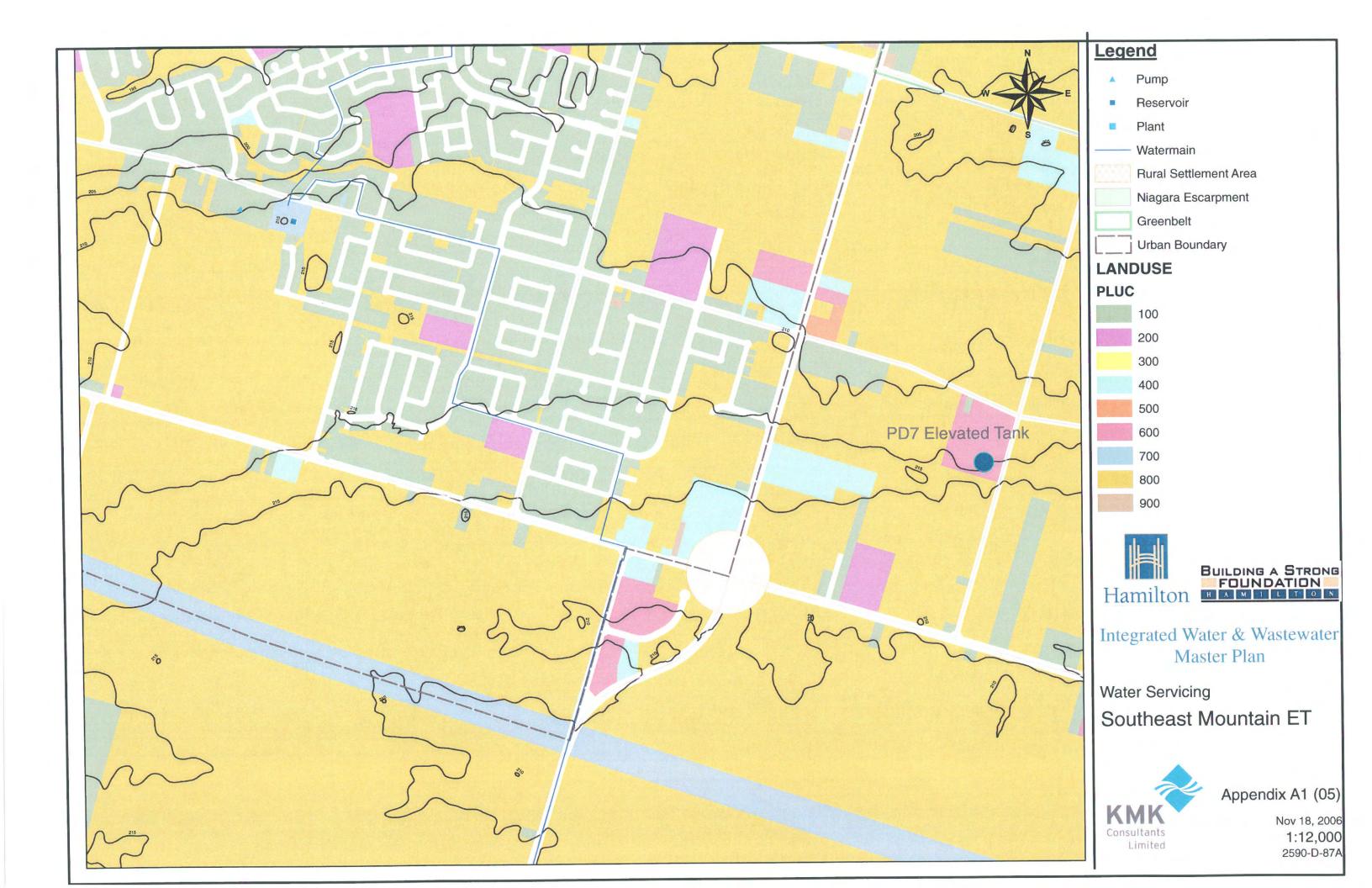
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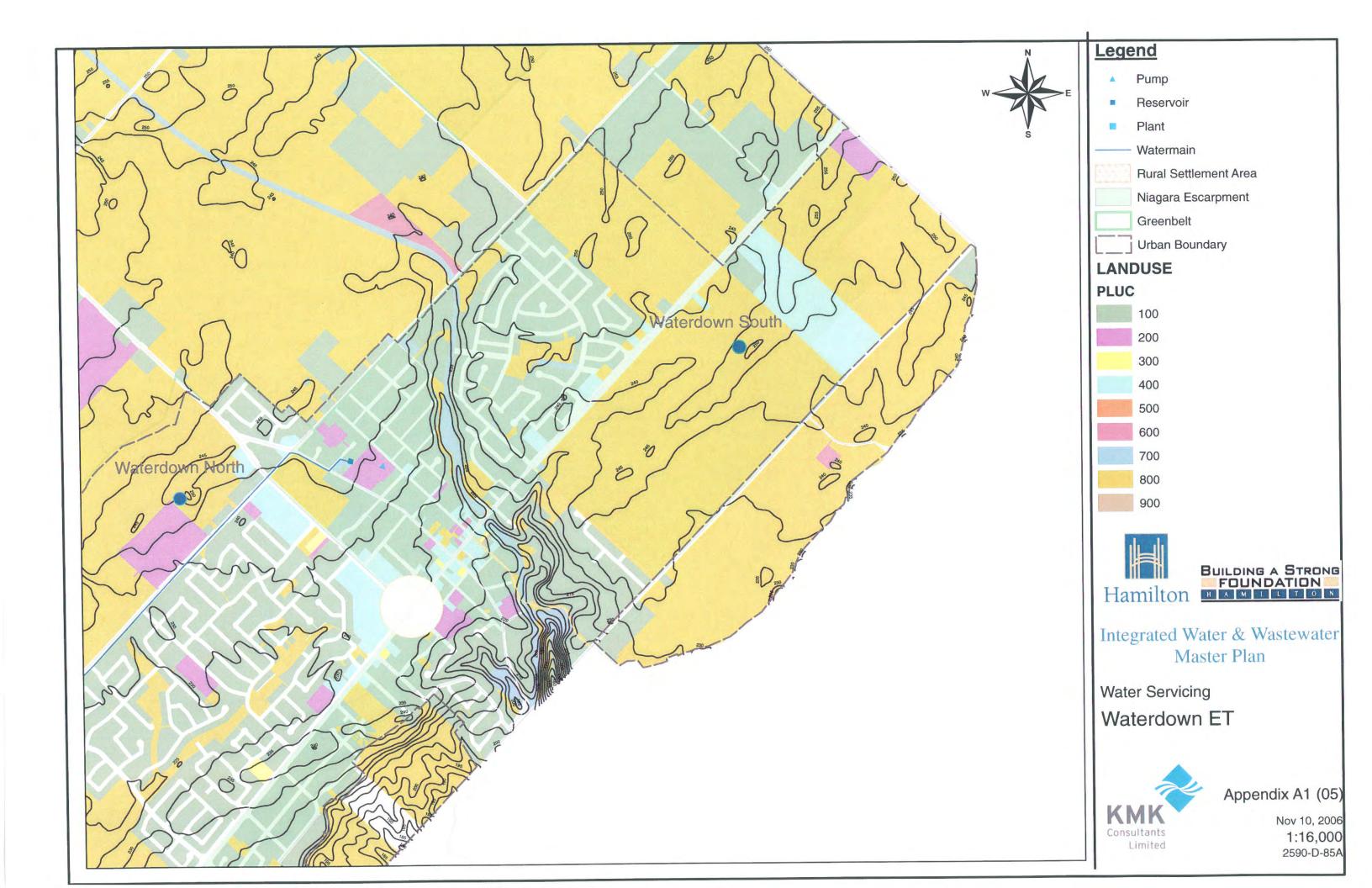
Appendix A-1 (05)

Storage Requirements

			ſ			As pe	r MOE Guideline	es					
Pressure District	Avg Day Demand (m3/d)	Max Day Demand (m3/d)	Equiv Pop	Fire (I/s)	Duration	Fire (m3)	Equalization (m3)	Emergency (m3)	Storage (m3)	Storage Max Day (m3)	Existing Storage (m3)	Surplus/ Deficit (ML)	Surplus/ Deficit Max Day (ML)
1	86,772.51	173,545.02	241,034.75	250	4	3,600.00	43,386.26	11,746.56	58,732.82	173,545.02		I	(1412)
4	14,051.61	28,103.23	39,032.26	250	4	3,600.00	7,025.81	2,656.45	13,282.26	28,103.23			
									72,015.08	201,648.25	360,276.79	288.26	158.63
2	37,068.46	74,136.91	102,967.93	250	4	3,600.00	18,534.23	5,533.56	27,667.79	74,136.91			
3	4,656.89	9,313.78		200	2	1,440.00	2,328.45	942.11	4,710.56	9,313.78			
17	909.47	1,818.94	2,526.31	100	2	720.00	454.73	293.68	1,468.42	1,818.94			
	•		· · · · · ·						33,846.76	85,269.64	129,103.67	95.26	43.83
5	30,242.59	60,485.18	84,007.19	250	4	3,600.00	15,121.29	4,680.32	23,401.62	60,485.18			
6	45,990.57	91,981.14	127,751.58	250	4	3,600.00	22,995.28	6,648.82	33,244.11	91,981.14			
									56,645.72	152,466.32	227,299.79	170.65	74.83
7	35,073.48	70,146.96	97,426.34	250		3,600.00	17,536.74	5,284.19	26,420.93	70,146.96			
23	5,070.51	10,141.03	14,084.76	250	4	3,600.00	2,535.26	1,533.81	7,669.07	10,141.03			
								L	34,090.00	80,287.99	11,371.65	-22.72	-68.92
9	35.10	70.20		38		273.60	17.55	72.79	363.94	70.20			
10	642.42	1,284.83	1,784.49	88	2	633.60	321.21	238.70	1,193.51	1,284.83			
									1,557.45	1,355.04	1,140.00	-0.42	-0.22
11	6,721.59	13,443.18	18,671.08	250	4	3,600.00	3,360.80	1,740.20	8,700.99	13,443.18			
21	41.89	83.79	116.37	38	2	273.60	20.95	73.64	368.18	83.79			
									9,069.18	13,526.97	16,372.23	7.30	2.85
12	2,004.29	4,008.57	5,567.46	160	3	1,728.00	1,002.14	682.54	3,412.68	4,008.57	3,700.96	0.29	-0.31
16	10,297.37	20,594.73	34,324.55	440	4	6,336.00	5,148.68	2,871.17	14,355.85	20,594.73			
19	146.64	293.28	407.34	38	4	547.20	73.32	155.13	775.65	293.28			
20	90.36	180.73	251.01	38	4	547.20	45.18	148.10	740.48	180.73			
									15,871.98	21,068.74	10,055.55	-5.82	-11.01
18	11,837.93	23,675.87	32,883.15	250		3,600.00		2,379.74	11,898.71	23,675.87			
13	265.21	530.43	736.70	38	2	273.60	132.61	101.55	507.76				
14	154.08	308.15	427.99	38	2	273.60	77.04	87.66	438.30	308.15			
									12,844.76	24,514.45	36,069.57	23.22	11.56
Total	292,072.98	584,145.96							239,353.61	584,145.96	795,390.22	556.0366	211.24









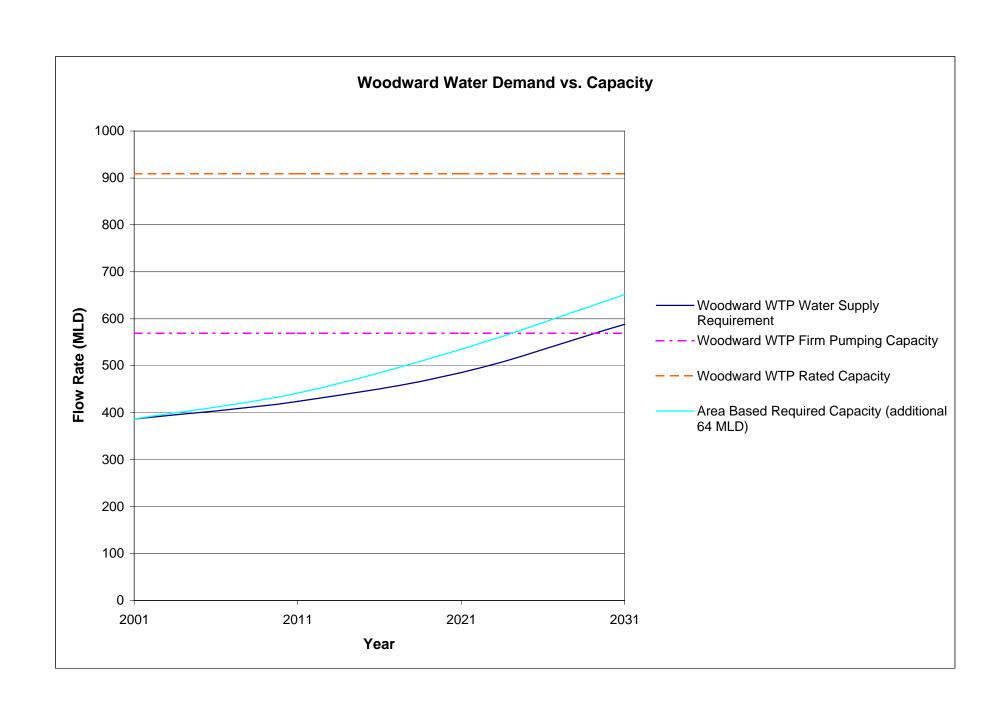




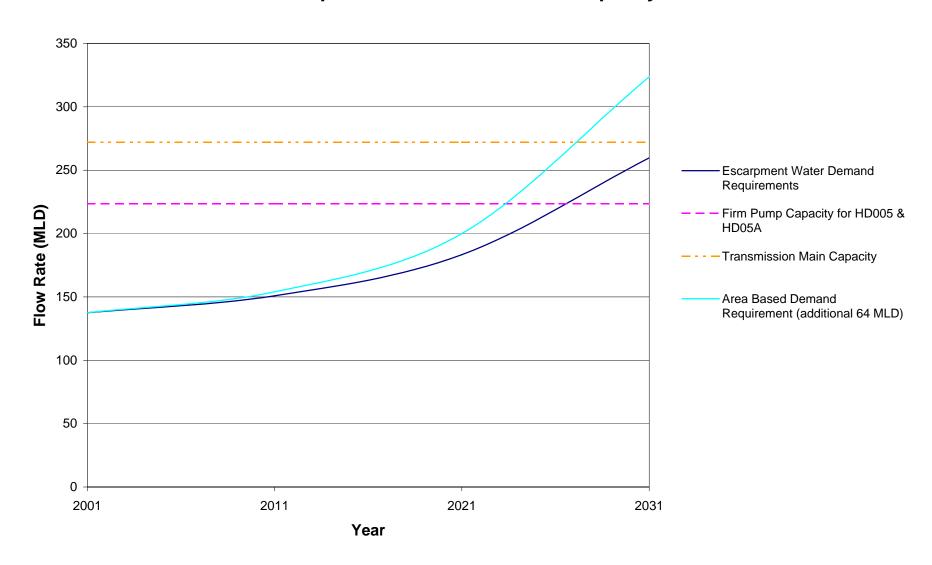
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Appendix A-1 (06)

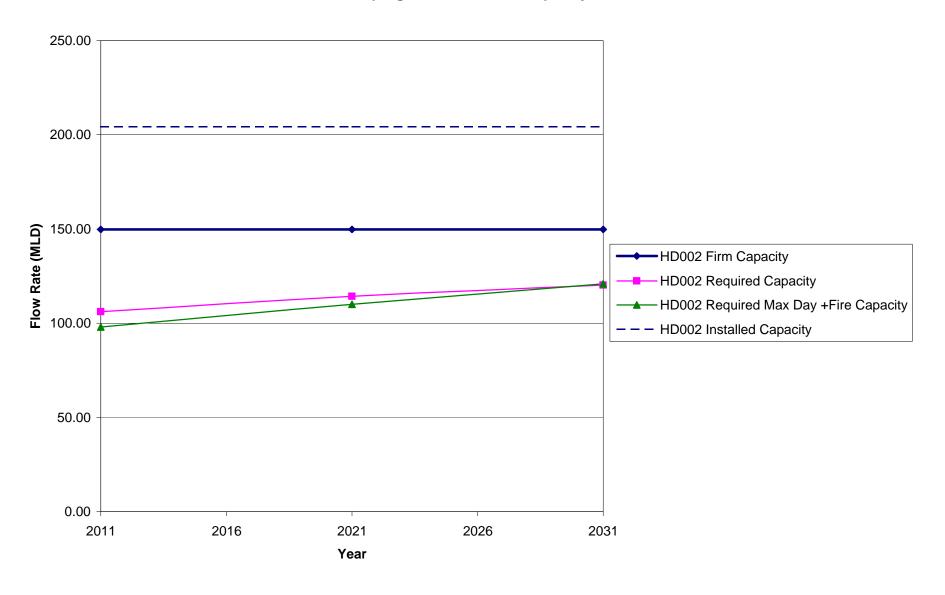
System Upgrade Requirements



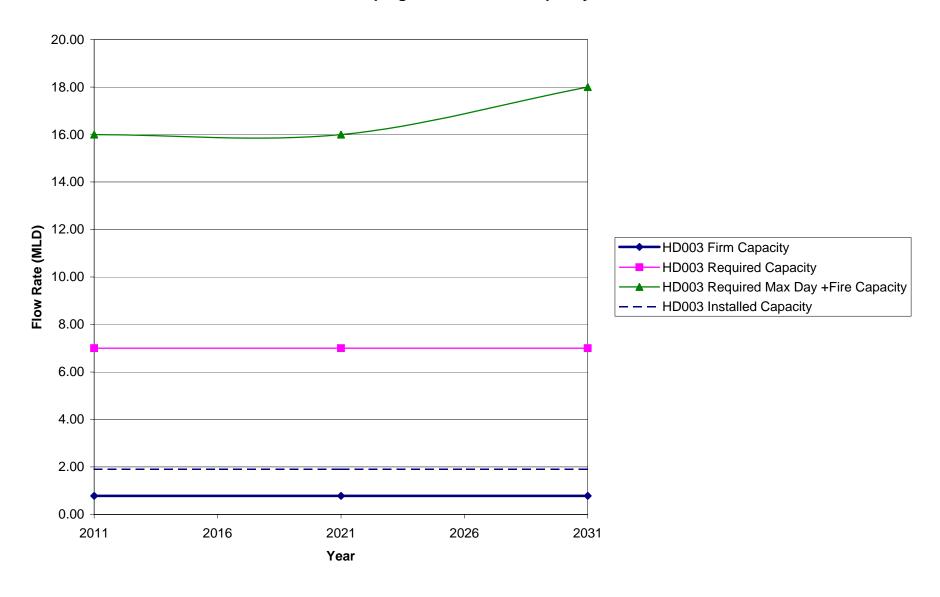
Escarpment Water Demand vs. Capacity



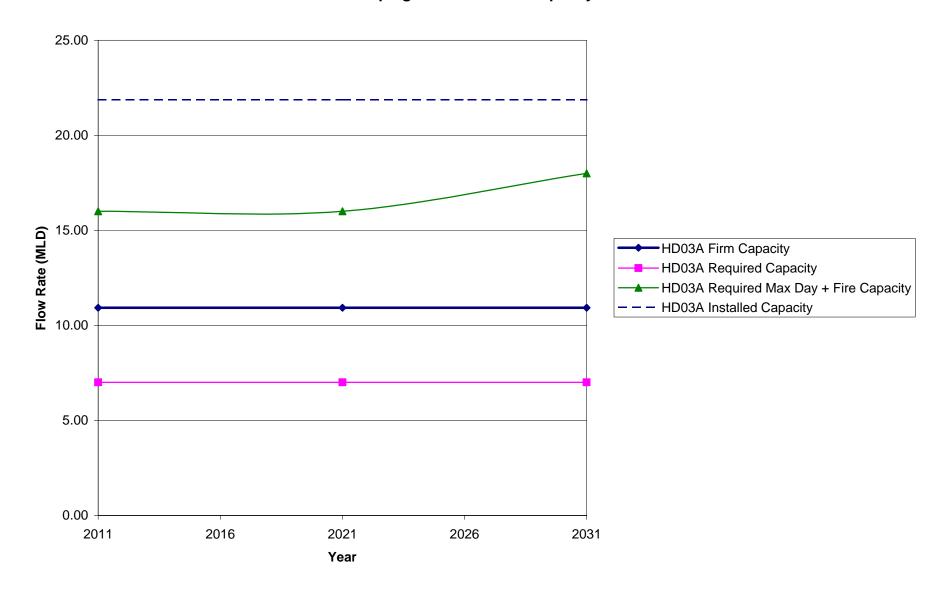
Pumping Station HD002 Capacity



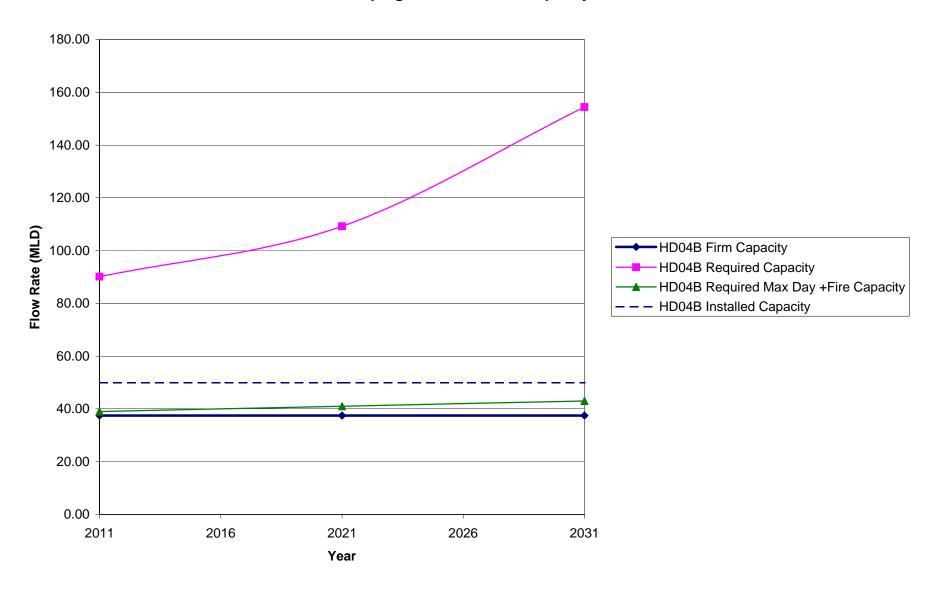
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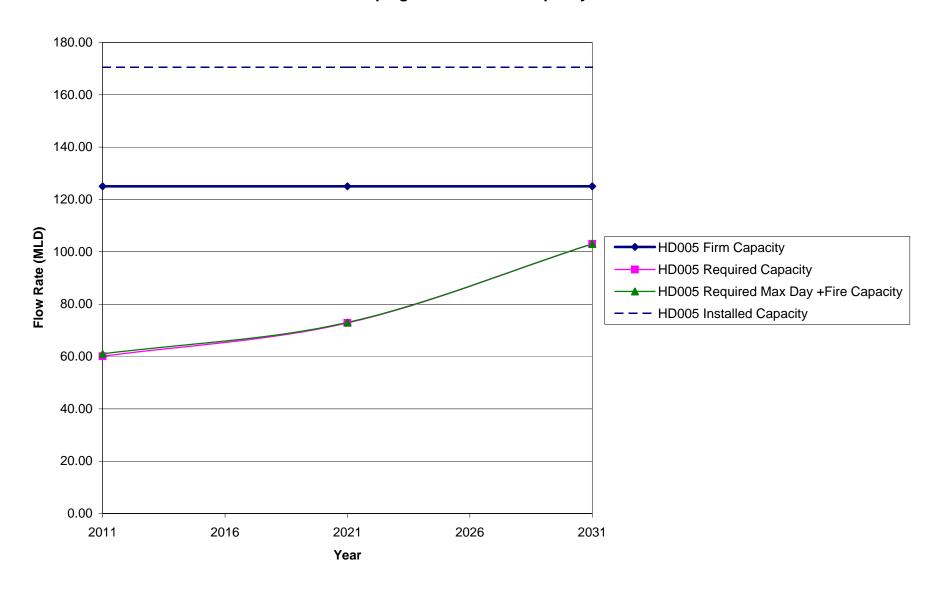
Pumping Station HD03A Capacity



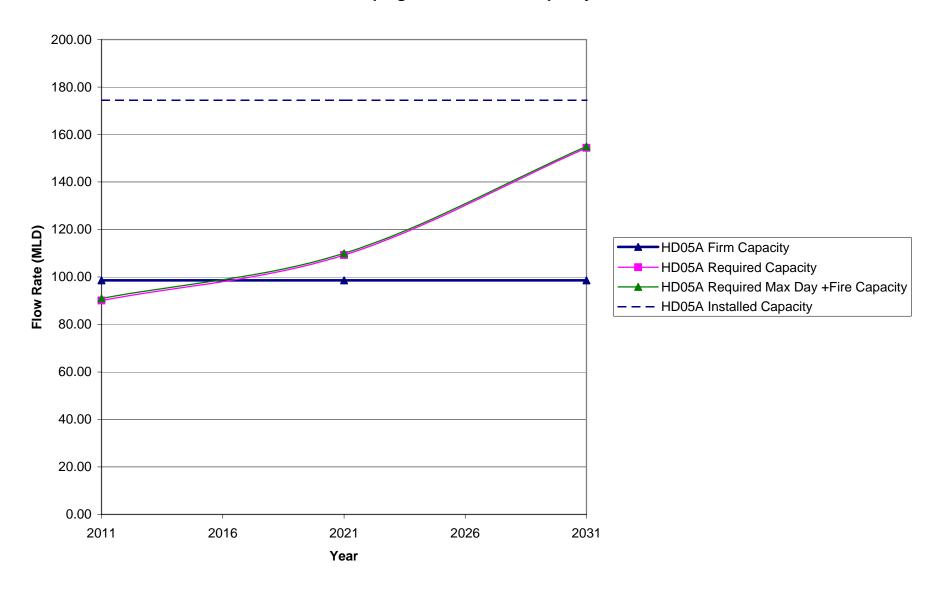
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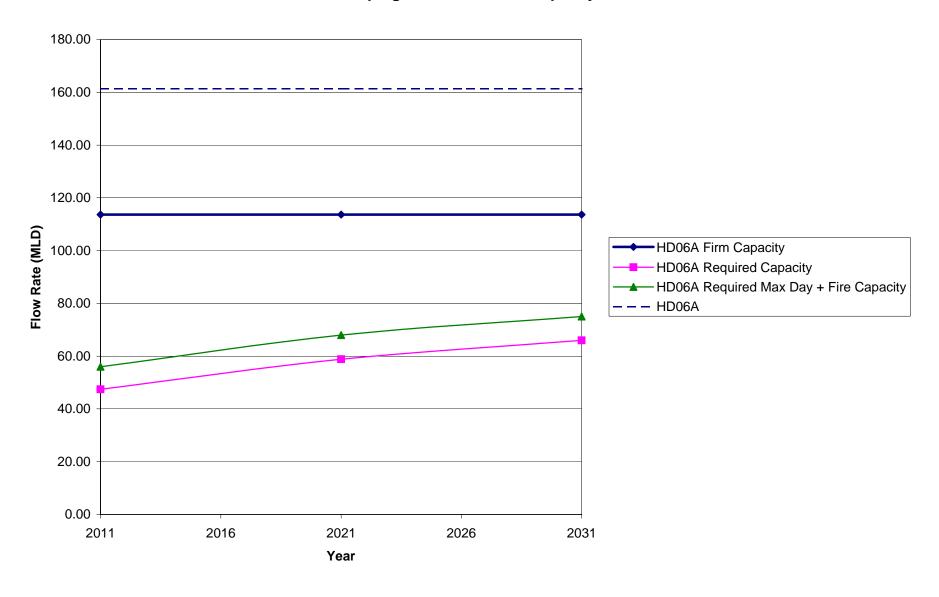
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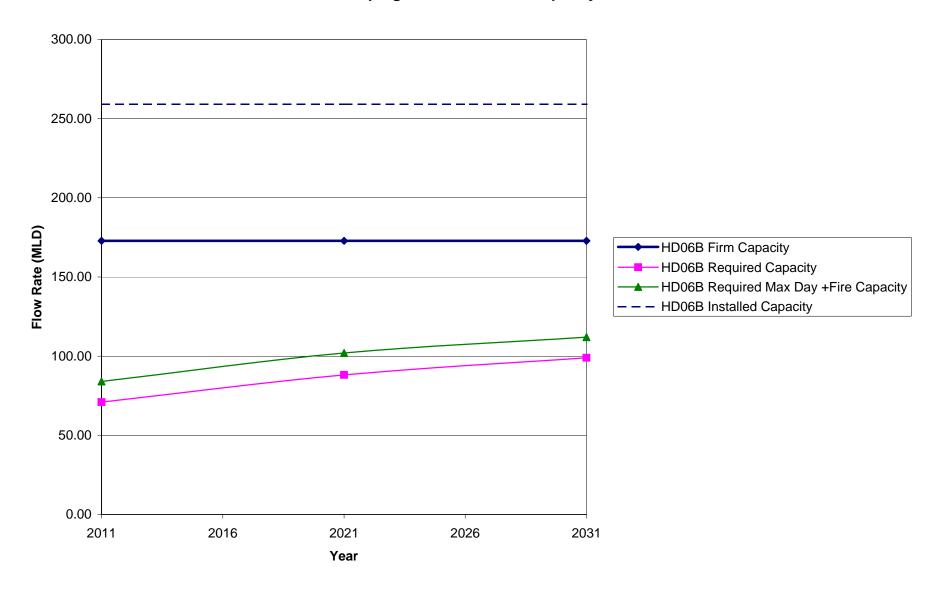
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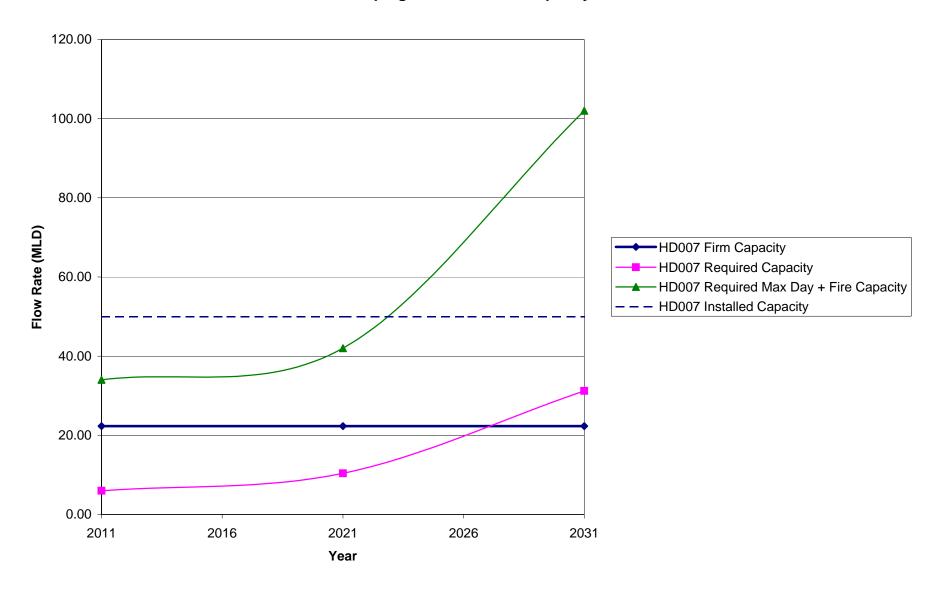
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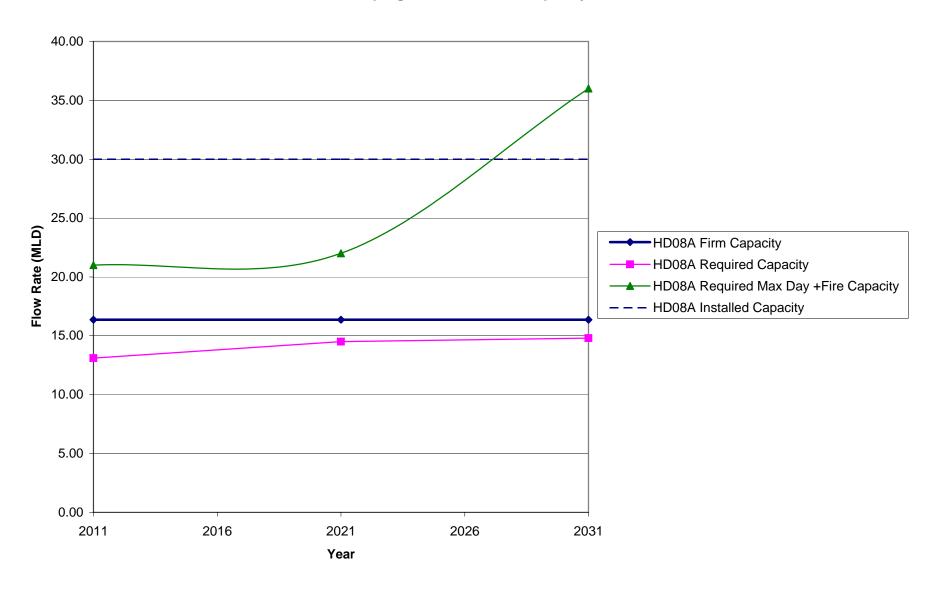
Pumping Station HD006B Capacity



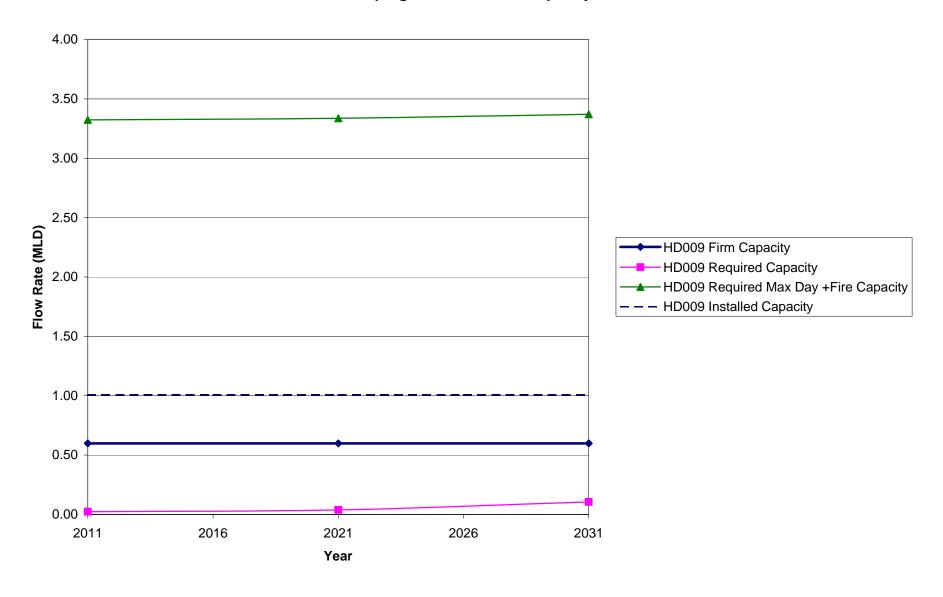
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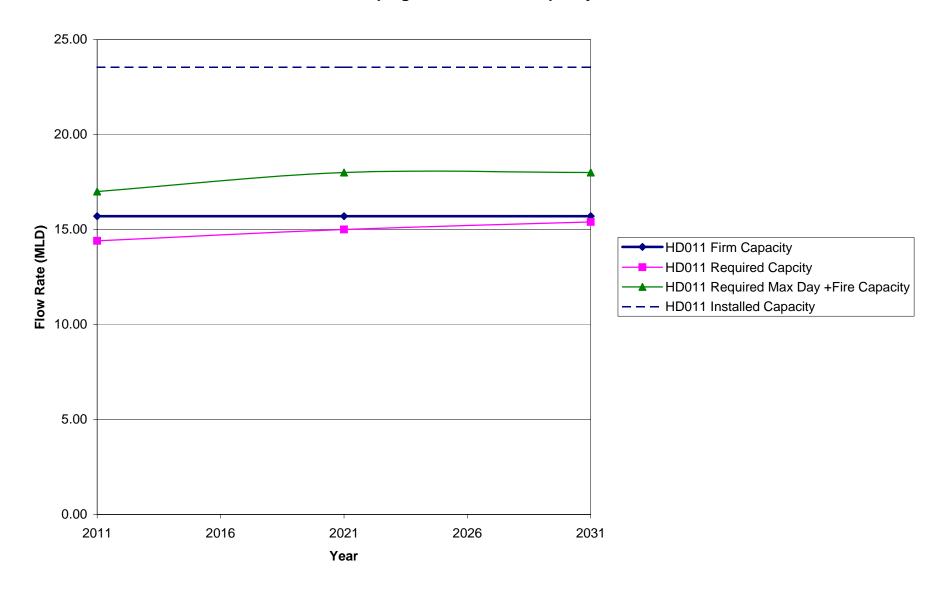
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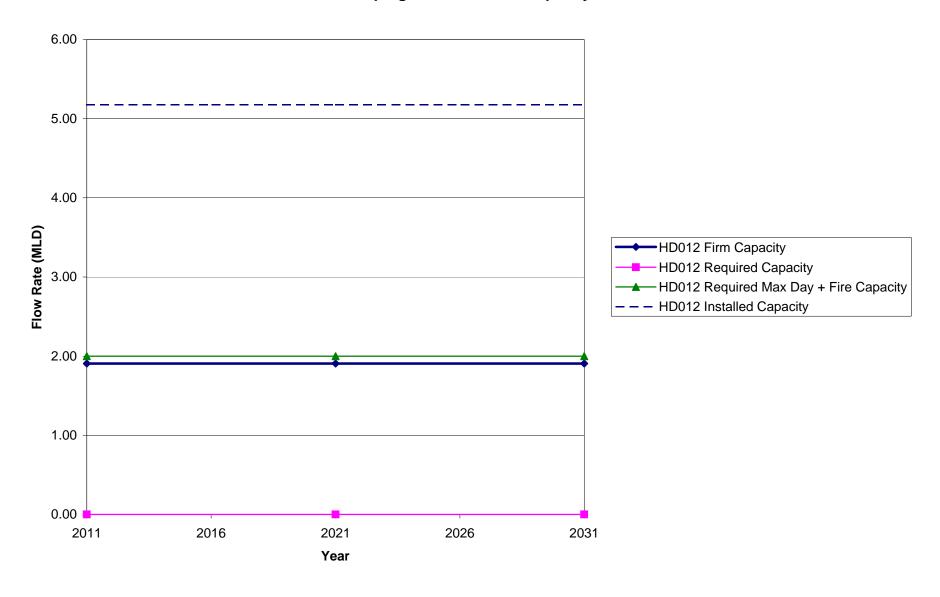
Pumping Station HD009 Capacity



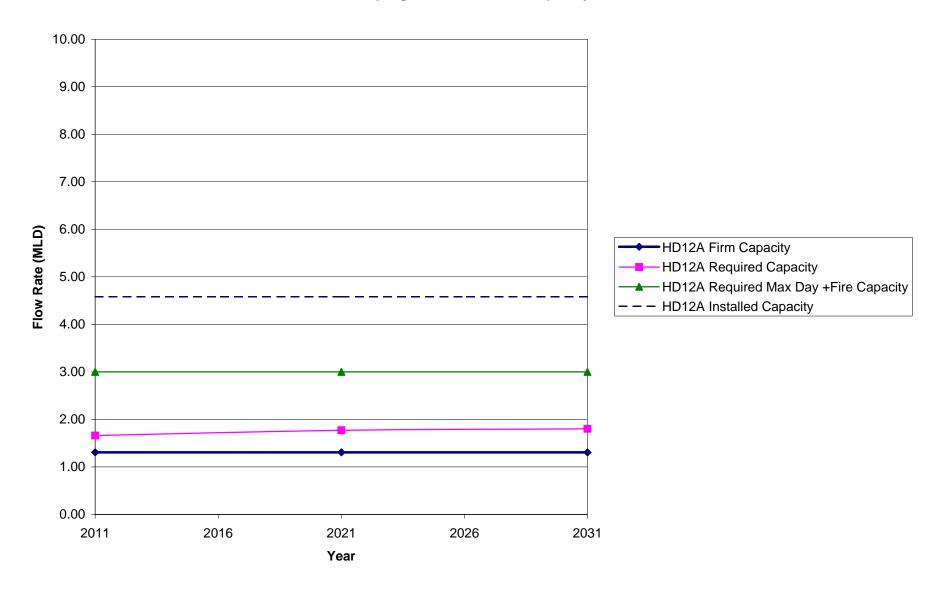
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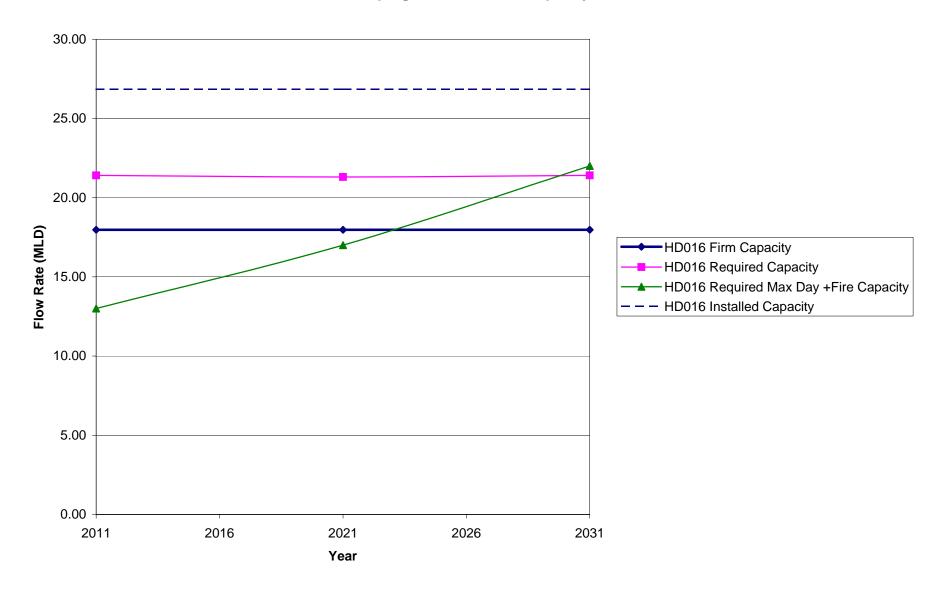
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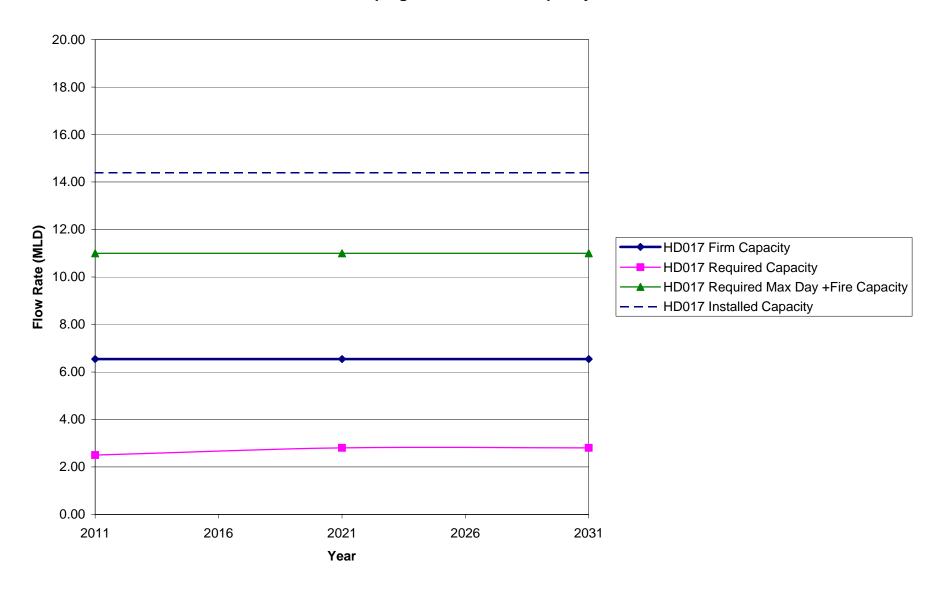
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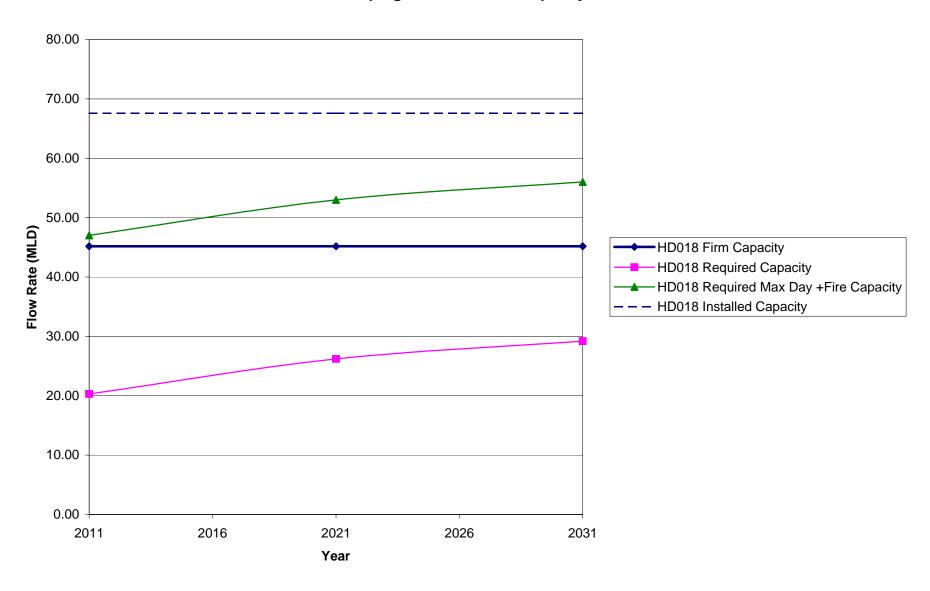
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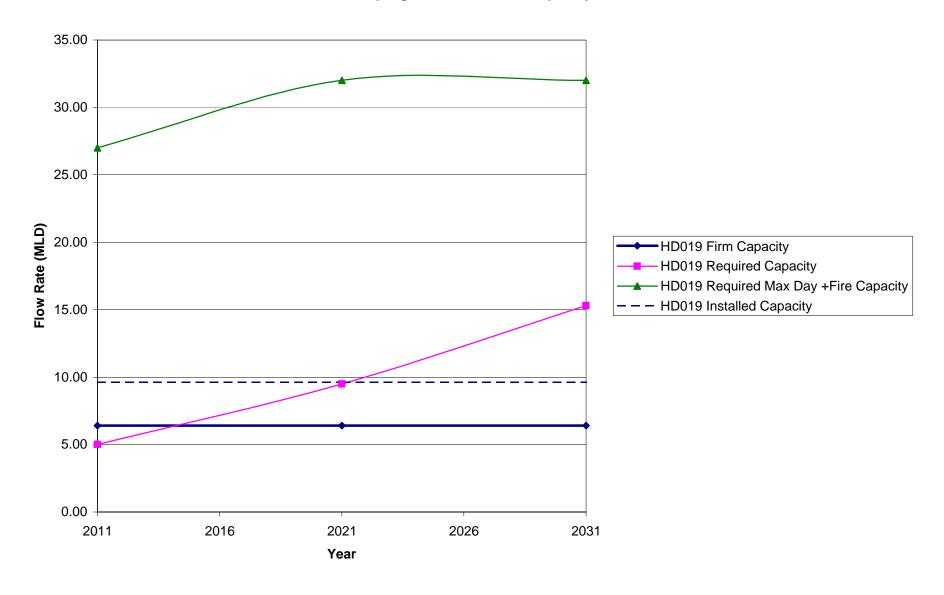
Pumping Station HD017 Capacity



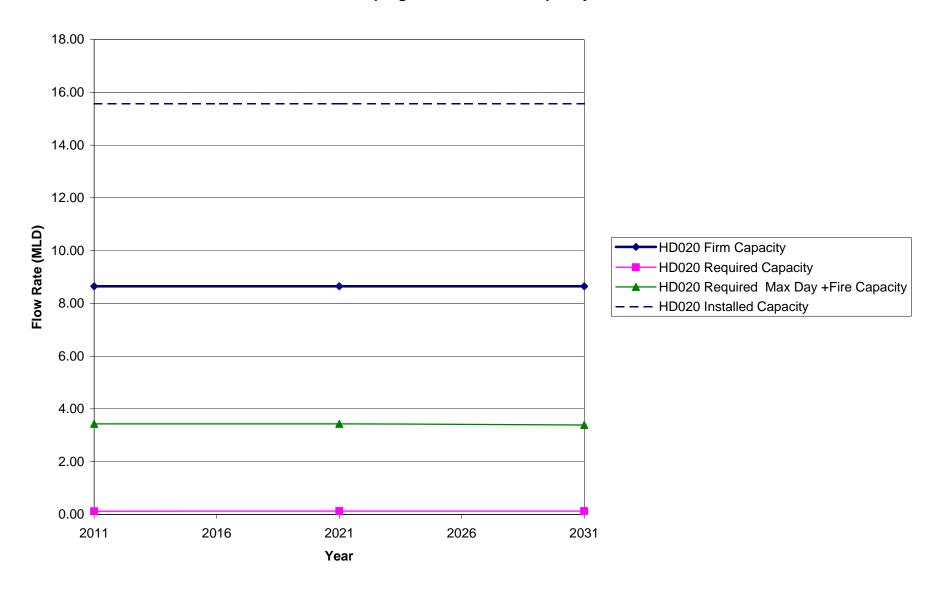
Pumping Station HD018 Capacity



Pumping Station HD019 Capacity



Pumping Station HD020 Capacity



Report III - Master Plan Class EA Report

Appendix A-1 (07)

Watermain Projects Alignment



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.





Water Servicing

W - 3 Watermain



22 NOV 06 1:10,000 2590-D-91



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases



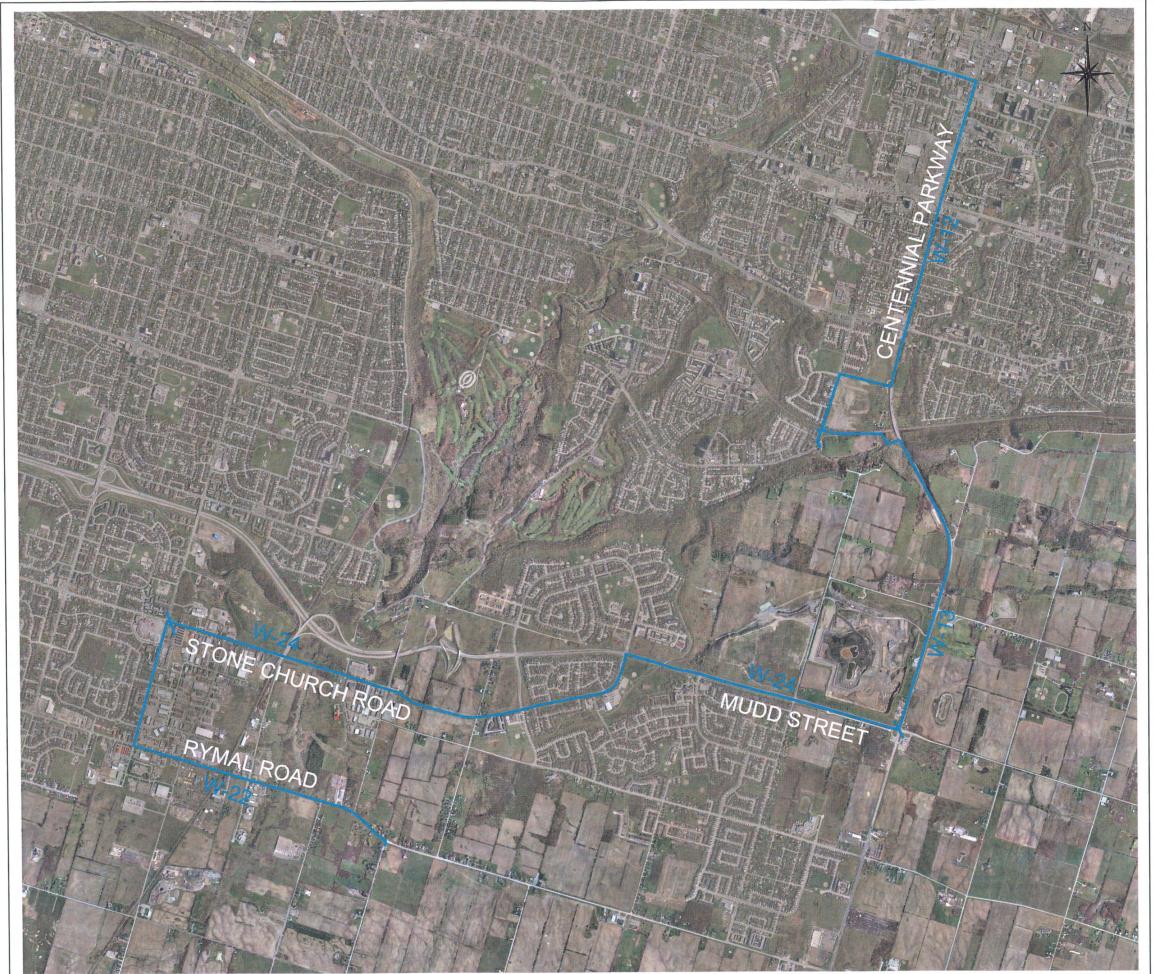


Water Servicing

W - 9 & 27 Watermain



22 NOV 06 1:30,000 2590-D-94



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.





Water Servicing

W - 12, 13, 22 & 24 Watermain



22 NOV 06 1:30,000 2590-D-95



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.





Water Servicing

W - 19 Watermain





Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases





Water Servicing

W - 25 Watermain





Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.





Water Servicing

W - 30 Watermain



22 NOV 06 1:20,000 2590-D-96







Report III - Master Plan Class EA Report

Appendix A-2

Wastewater Servicing Analysis







Report III - Master Plan Class EA Report

Appendix A-2 (01)

Projects WW-01 and WW-02

Twenty Road SPS (HC018) and Forcemain Upgrades



To: City of Hamilton

Lisa De Angelis, Udo Ehrenberg

From: KMK Consultants Limited

Kevin Brown

CC: Chris Hamel, KMK Consultants Limited

RE: Twenty Road Sewage Pumping Station

Project: Hamilton Water and Wastewater Master Plan

Date: 17 February 2006

The City has requested that KMK complete a capacity evaluation of the key wastewater servicing infrastructure associated with the Twenty Road Sewage Pumping Station (SPS).

The City provided KMK with the details on the existing infrastructure as well as SCADA data.

The Twenty Road SPS currently services an area generally bound by:

- Twenty Road to the north;
- Whitechurch Road to the South;
- Glancaster Road to the west; and,
- Upper James Street/Highway 6 to the east.

The Twenty Road SPS is experiencing excessive inflows during wet weather events, and is approaching or exceeding the existing design capacity. The station currently has two constant-speed pumps installed, with space to add a third.

The existing pumps are rated at 88 L/s each, according to a station summary report prepared by the City in the summer of 2001. KMK reviewed the pump manufacturer data for the Wemco-Hydrostal model F6K-H, 13.5-inch impellor, 1750 RPM pump. KMK also attempted to confirm the current operating point of the existing pumps by examining SCADA data from the period of December 18th 2005 through January 15th 2006, but no correlation between pump run times and station discharge could be determined. It is recommended that the City review how data is collected at this station.

As such, part of our analysis is based on our derived theoretical system curve and manufacturer pump curves. Under existing conditions with the existing 300 mm diameter forcemain, the existing and potential capacity of the SPS would be as follows:

- 130 L/s with one existing pump in operation;
- 170 L/s with both existing pumps in operation; and,
- 190 L/s with both existing and one future pump in operation.



Adding a third pump would therefore increase the firm capacity of the station from 130 L/s to 170 L/s. Due to the very high forcemain losses, however, the total capacity of the station would only increase from 170 L/s to 190 L/s. SCADA data have indicated that there have been periods when the two existing pumps were operating simultaneously, so adding a third pump would therefore not provide sufficient additional benefit under periods of excessive inflow.

Design drawings of the station indicate that provision was made for the eventual twinning of the existing 300 mm forcemain. Doing so would increase the station capacity, as the hydraulic losses would be reduced. Adding a third pump and twinning the forcemain would result in the following station capacities:

- 170 L/s with one pump in operation;
- 250 L/s with both pumps in operation (firm capacity); and,
- 290 L/s with all three pumps in operation (total capacity).

As noted from the potential capacities above, twinning the forcemain provides the greatest benefit under existing conditions as well as future conditions.

The recommendations to address the station limitations are as follows:

- 1. Install a twinned forcemain from the existing SPS to the existing MH HG22A050 on Upper James Street, north of Alderson Drive.
- 2. Install a third pump. It is recommended to have firm capacity between two pumps with a third pump as standby.
- 3. Inspect the existing appurtenances in the meter chamber. Based on the discrepancies with the SCADA data, it is recommended that installation of a flow meter and potentially a pressure gauge be considered.

The City could consider phasing the above noted recommendations with installation of the twinned forcemain as the first priority.

Based on review of the existing conditions of the SPS, it should be noted that space for the future third pump has been provided. However, the MCC does not appear to have made allowance for the third starter. Further review of the electrical impacts should be undertaken.





Approximate cost estimates for the upgrades are provided below:

•	Total:	\$550,000
•	Engineering and Contingency (10%):	\$50,000
•	New flow meters:	\$50,000
•	New pump and starter:	\$90,000
•	900 m of 300 mm diameter forcemain (\$400/m):	\$360,000

We trust the above summary is satisfactory. Should you have any comments or questions, please contact the undersigned or Chris Hamel.

Yours truly,

KMK CONSULTANTS LIMITED

-Kevin







Report III - Master Plan Class EA Report

Appendix A-2 (02)

Projects WW-03, WW-04, and WW-05

Harmony Hall SPS (HC008) and Forcemain Upgrades



To: City of Hamilton

Udo Ehrenberg

From: KMK Consultants Limited

Kevin Brown

cc: Lisa De Angelis – City of Hamilton

Chris Hamel – KMK Consultants Limited

Re: ORC Lands at Highway 6 and Highway 403

Project: Hamilton Water and Wastewater System - Analysis Support

Date: September 25, 2006

Udo:

We have completed an analysis of the Southcote Road sewer in order to determine what upgrades would be required to service the ORC Lands southeast of the Highway 6 interchange on Highway 403.

Earlier this year, we performed a similar analysis for the Southcote Woodlands Development Application. This analysis assumed a population density of 3.0 persons/unit, and an average flow of 360 Lpcd. The analysis concluded that the existing Harmony Hall SPS, the existing forcemain, and the Southcote Road gravity sewer all had sufficient capacity for the proposed development of 150 units, but that the SPS would need to be upgraded to 18.9 L/s in order to accommodate the ultimate development of that site. These results were presented to the City in a memorandum dated February 17, 2006.

That earlier analysis did not consider the ultimate development of the ORC Lands immediately to the west of Southcote Woodlands, nor the undeveloped land to the South of Garner Road. At the time, it was assumed that these lands would ultimately drain to the west to the Calvin Street SPS. Since the City is planning on replacing the existing Harmony Hall SPS with a completely new facility, we have now expanded upon that earlier analysis to also include the ORC Lands.

According to the Draft Plan of Subdivision for the ORC Lands, 681 additional housing units are planned, including some mixed-use development adjacent to Garner Road and a school. The basis for the flow estimates are as follows:

- The residential flows were evaluated assuming an average flow of 360 Lpcd and a unit density of 3 persons/unit.
- An average flow allowance of 28 m³/ha·d was included for the school and the commercial portion of the mixed-use area.
- An external infiltration allowance of 0.2L/s·ha was included.

The details of the design flow calculation are presented in Table 1.



Inclusion of the ORC Lands in the new SPS would result in a required firm capacity of 60.8 L/s. The existing sewage forcemain only has a capacity of 40 L/s, so it would also need to be replaced.

Our February 2006 memo indicated that the section of gravity sewer along Southcote Road immediately downstream of where the forcemain currently discharges has a capacity of only 60 L/s, and that the Southcote sewer will eventually be extended to service some additional properties to the south. An analysis of the entire stretch of gravity sewers was performed in order to assess the impact of any increased flows from the proposed SPS to the point where the gravity sewer discharges into the Wilson Street Trunk Sewer. These calculations are included in Table 2.

The sewer capacity analysis indicates that the first four sections of the Southcote Road sewer downstream of where the existing forcemain discharges would not have sufficient capacity to accept the increased SPS flows. These sections – totalling approximately 400 m in length – could be bypassed by extending an upgraded forcemain farther north on Southcote Road, to manhole AM10A003. The only other section of sewer where surcharging could be an issue is the 27 m section between manholes AM08A007 and AL08A001. The analysis indicates that design flows would exceed that pipe's capacity by approximately 22%.

A preliminary analysis of potential development south of Garner Road was also undertaken to determine whether there is any merit in servicing those lands through the Harmony Hall SPS. Based on the GRIDS planning projections, that additional catchment area – measuring 75.22 ha – could see an additional 1,000 jobs added between now and 2031. This would require an additional 27 L/s of firm pumping capacity (including the infiltration allowance) if these lands were also serviced through the Harmony Hall SPS. This additional flow would result in another two sections of downstream sewers requiring upgrades; 57 m between manholes AM09A008 and AM09A012, and 131 m between manholes AL08A049 and AL08A051

Replacing the three sections of undersized sewer that we have identified would cost approximately \$300,000, and the new forcemain would cost approximately \$500,000. These costs are in addition to the \$3M that was identified in the Master Plan to replace the existing Harmony Hall SPS.

While we have not performed a full analysis of the required upgrades in order to service the ORC Lands and the lands south of Garner Road through the Calvin Street SPS, we anticipate that constructing a new sewer under Highway 6 and upgrading the Calvin Street SPS and its receiving sewers would prove to be even more costly.

-Kevin



^{tel} **905 459 4780** fax 905 459 7869 web www.kmk.ca

Table 1 – Wastewater Flow to the Harmony Hall Pumping Station

	Existing Serviced Development	Existing and Southcote Woodlands	Existing, Southcote Woodlands and ORC Lands
Number of Residential Units			
Existing Development	30	30	30
Southcote Woodlands – Ultimate	0	186	186
ORC Lands - Ultimate	0	0	681
TOTAL	30	216	897
Commercial/Institutional Development			
School	0	0	2.50 ha
Mixed-Use Commercial	0	0	1.30 ha
TOTAL	0	0	3.80 ha
Design Population			
Population (Assuming 3 persons/unit)	90	648	2,691
Design Flow			
Average Residential Flow (360 Lpcd)	0.38 L/s	2.70 L/s	11.21 L/s
Average ICI Flow (28 m³/ha-d)	0	0	1.23 L/s
Peaking Factor, M=5/[(C/1000)^0.2]	5.0	5.0	4.1
Domestic Peak Flow	1.9 L/s	13.5 L/s	51.0 L/s
Area	6.36 ha	26.97 ha	49.01 ha
External Infiltration (Area x 0.2 L/ha/s)	1.3 L/s	5.4 L/s	9.8 L/s
TOTAL PEAK FLOW, (G) + (I)	3.2 L/s	18.9 L/s	60.8 L/s

City of Hamilton Sanitary Sewer Design

Location: Southcote Sewer - Harmony Hall SPS Upgraded to 60.8 L/s

KMK 2874.01

Project #: Checked by: Computed by: Date: KCB MZ 25-Sep-06

Infiltration Factor, F_i= Maximum Allowable Flow=

Design Flow Factor, F= 0.00417 L/s (equivalent to 360 Lpcd)

0.2 L/s/ha

100 % of sewer's design capacity

Maximum Velocity= 3.0 m/s Minimum Velocity= 0.6 m/s

		A	F	То	Pop	Area	(ha)	Popu	lation	Dankina	Sanitary F	low (L/s)	Infiltration	Additional	Total Flow		-	Proposed S	ewer Desigr	1		Check	Design
	Location	Area Number	From MH	MH	Density (pers/ha)	Incremental	Cumulative	Incremental	Cumulative	Peaking Factor	Average	Peak	Flow (L/s)	Flow (L/s)	(L/s)	Dia. (mm)	Grade (%)	Mann. N	Velocity (m/s)	Qd Full (L/s)	% Capacity	Velocity	Capacity
1	Harmony Hall FM	AM11	PS02	L002	(pers/ria)	0.001	0.00	0	0	5.00	0.00	0.00	0.00	60.80	60.80	200	1.00	0.015	0.90	28.43	214	OK	UNDER
2	Southcote Road	AN10 AN10	A002 A001	A001 A001	62 62	10.53 0.96	10.53 11.49	653 60	653 712	5.00 5.00	2.72 2.97	13.61 14.85	2.11 2.30	0.00	76.52 77.95	250 250	1.20 0.96	0.015 0.015	1.15 1.03	56.46 50.50	136 154	OK OK	UNDER UNDER
4		AM10	A001	A001	60	3.32	14.81	199	912	5.00	3.80	19.01	2.96	0.00	82.77	300	0.45	0.015	0.80	56.23	147	OK	UNDER
5		AM10	A002	A003	60	0.78	15.59	47	958	5.00	4.00	19.98	3.12	0.00	83.90	300	0.61	0.015	0.93	65.46	128	OK	UNDER
6		AM10 AM10	A003 A004	A004 A005	60 60	0.77 0.62	16.36 16.98	46 37	1005 1042	5.00 4.96	4.19 4.34	20.93 21.54	3.27 3.40	0.00	85.00 85.74	350 350	1.00 1.11	0.015 0.015	1.31 1.38	126.43 133.21	67 64	OK OK	OK OK
8		AM10	A005	A001	60	12.66	29.64	760	1801	4.44	7.51	33.39	5.93	0.00	100.12	525	0.50	0.015	1.22	263.59	38	OK	OK
9 10		AM09 AM09	A001 A002	A002 A079	62 0	4.21 0.00	33.85 33.85	261 0	2062 2062	4.33 4.33	8.60 8.60	37.21 37.21	6.77 6.77	0.00	104.78 104.78	525 525	0.39 1.42	0.015 0.015	1.08 2.05	232.79 444.21	45 24	OK OK	OK OK
11		AM09	A079	A003	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	104.78	450	1.42	0.015	1.85	294.48	36	OK	OK
12		AM09	A003	A004	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	104.78	450	0.37	0.015	0.95	150.32	70	OK	OK
13 14		AM09 AM09	A004 A005	A005 A006	0	0.00 0.00	33.85 33.85	0 0	2062 2062	4.33 4.33	8.60 8.60	37.21 37.21	6.77 6.77	0.00	104.78 104.78	450 525	0.34 0.38	0.015 0.015	0.91 1.06	144.10 229.79	73 46	OK OK	OK OK
15		AM09	A006	A007	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	104.78	450	0.33	0.015	0.89	141.96	74	OK	OK
16 17	Dorval Drive	AM09 AM09	A007 A008	A008 A012	60 60	4.08 5.05	37.93 42.98	245 303	2307 2610	4.23 4.13	9.62 10.88	40.70 44.92	7.59 8.60	0.00	109.08 114.32	450 450	0.53 0.24	0.015 0.015	1.13 0.76	179.91 121.07	61 94	OK OK	OK OK
18	Elm Hill Boulevard	AM09	A012	A012	60	0.89	43.87	53	2664	4.13	11.11	45.65	8.77	0.00	115.23	375	1.15	0.015	1.48	162.97	71	OK	OK
19		AM09	A011	A009	60	0.24	44.11	14	2678	4.11	11.17	45.85	8.82	0.00	115.47	375	0.97	0.015	1.36	149.68	77	OK	OK
20 21		AM09 AM08	A009 A005	A005 A006	60 60	1.02 0.81	45.13 45.94	61 49	2739 2788	4.09 4.07	11.42 11.63	46.69 47.35	9.03 9.19	0.00	116.51 117.34	375 375	0.96 1.05	0.015 0.015	1.35 1.41	148.90 155.73	78 75	OK OK	OK OK
22	Golf Links Road	AM08	A006	A007	60	3.26	49.20	196	2983	4.02	12.44	49.99	9.84	0.00	120.63	450	0.58	0.015	1.18	188.20	64	OK	OK
23 24		AM08 AL08	A007 A001	A001 A020	0 60	0.00 13.24	49.20 62.44	0 794	2983 3778	4.02 3.83	12.44 15.75	49.99 60.38	9.84 12.49	0.00	120.63 133.67	450 450	0.16 1.00	0.015 0.015	0.62 1.55	98.85 247.12	122 54	OK OK	UNDER OK
25		AL09	A020	A019	60	0.50	62.94	30	3808	3.83	15.88	60.76	12.59	0.00	134.15	525	0.21	0.015	0.79	170.82	79	OK	OK
26 27	" Focoment	AL09 AL09	A019 A015	A015 A014	60 60	9.82 9.93	72.76 82.69	589	4397 4993	3.72 3.62	18.34	68.18 75.47	14.55 16.54	0.00	143.53 152.81	525 600	0.34 0.92	0.015 0.013	1.00 2.08	217.36 589.02	66 26	OK OK	OK OK
28	Easement	AL09 AL09	A015	A014 A048	0	1.34	84.03	596 0	4993	3.62	20.82 20.82	75.47	16.81	0.00	153.08	600	0.92	0.013	0.95	267.68	57	OK	OK
29		AL08	A048	A049	23	5.34	89.37	123	5116	3.61	21.33	76.95	17.87	0.00	155.63	600	0.17	0.013	0.90	253.20	61	OK	OK
30 31	-	AL08	A049	A051	0	3.88	93.25	0	5116	3.61	21.33	76.95	18.65	0.00	156.40	600	0.07	0.013	0.57	162.47	96	LOW	OK
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City of Hamilton Sanitary Sewer Design

Location: Southcote Sewer - Harmony Hall SPS Upgraded to 87.8 L/s

Project #: Checked by: Computed by: KMK 2874.01

KCB MZ 25-Sep-06

Infiltration Factor, F_i= Maximum Allowable Flow=

Design Flow Factor, F= 0.00417 L/s (equivalent to 360 Lpcd)

0.2 L/s/ha

100 % of sewer's design capacity

Maximum Velocity= 3.0 m/s Minimum Velocity= 0.6 m/s

Date	·-	25-3ep-00							William	um velocity=	0.0	111/3											
		Area	From	To	Pop	Area	(ha)	Popu	lation	Peaking	Sanitary F	low (L/s)	Infiltration	Additional	Total Flow			Proposed S	ewer Desigr		1	Check	Design
	Location	Number	MH	MH	Density (pers/ha)	Incremental	Cumulative	Incremental	Cumulative	Factor	Average	Peak	Flow (L/s)		(L/s)	Dia. (mm)	Grade (%)	Mann. N	Velocity (m/s)	Qd Full (L/s)	% Capacity	Velocity	Capacity
1	Harmony Hall FM	AM11	PS02	L002	0	0.001	0.00	0	0	5.00	0.00	0.00	0.00	87.80	87.80	200	1.00	0.015	0.90	28.43	309	OK	UNDER
2	Southcote Road	AN10	A002	A001	62	10.53	10.53	653	653	5.00	2.72	13.61	2.11	0.00	103.52	250	1.20	0.015	1.15	56.46	183	OK	UNDER
3		AN10 AM10	A001 A001	A001 A002	62 60	0.96 3.32	11.49 14.81	60 199	712 912	5.00 5.00	2.97 3.80	14.85 19.01	2.30 2.96	0.00 0.00	104.95 109.77	250 300	0.96 0.45	0.015 0.015	1.03 0.80	50.50 56.23	208 195	OK OK	UNDER UNDER
5		AM10	A001	A002	60	0.78	15.59	47	958	5.00	4.00	19.98	3.12	0.00	110.90	300	0.43	0.015	0.93	65.46	169	OK	UNDER
6	"	AM10	A003	A004	60	0.77	16.36	46	1005	5.00	4.19	20.93	3.27	0.00	112.00	350	1.00	0.015	1.31	126.43	89	OK	OK
7 8		AM10 AM10	A004 A005	A005 A001	60 60	0.62 12.66	16.98 29.64	37 760	1042 1801	4.96 4.44	4.34 7.51	21.54 33.39	3.40 5.93	0.00 0.00	112.74 127.12	350 525	1.11 0.50	0.015 0.015	1.38 1.22	133.21 263.59	85 48	OK OK	OK OK
9		AM09	A001	A002	62	4.21	33.85	261	2062	4.33	8.60	37.21	6.77	0.00	131.78	525	0.39	0.015	1.08	232.79	57	OK	OK
10		AM09	A002	A079	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	131.78	525	1.42	0.015	2.05	444.21	30	OK	OK
11 12		AM09 AM09	A079 A003	A003 A004	0	0.00 0.00	33.85 33.85	0	2062 2062	4.33 4.33	8.60 8.60	37.21 37.21	6.77 6.77	0.00 0.00	131.78 131.78	450 450	1.42 0.37	0.015 0.015	1.85 0.95	294.48 150.32	45 88	OK OK	OK OK
13		AM09	A004	A005	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	131.78	450	0.34	0.015	0.91	144.10	91	OK	OK
14		AM09	A005	A006	0	0.00	33.85	0	2062	4.33	8.60	37.21	6.77	0.00	131.78	525	0.38	0.015	1.06	229.79	57	OK	OK
15 16	" Dorval Drive	AM09 AM09	A006 A007	A007 A008	0 60	0.00 4.08	33.85 37.93	0 245	2062 2307	4.33 4.23	8.60 9.62	37.21 40.70	6.77 7.59	0.00 0.00	131.78 136.08	450 450	0.33 0.53	0.015 0.015	0.89 1.13	141.96 179.91	93 76	OK OK	OK OK
17	"	AM09	A008	A012	60	5.05	42.98	303	2610	4.13	10.88	44.92	8.60	0.00	141.32	450	0.24	0.015	0.76	121.07	117	OK	UNDER
18	Elm Hill Boulevard	AM09	A012	A011	60	0.89	43.87	53	2664	4.11	11.11	45.65	8.77	0.00	142.23	375	1.15	0.015	1.48	162.97	87	OK	OK
19 20		AM09 AM09	A011 A009	A009 A005	60 60	0.24 1.02	44.11 45.13	14 61	2678 2739	4.11 4.09	11.17 11.42	45.85 46.69	8.82 9.03	0.00 0.00	142.47 143.51	375 375	0.97 0.96	0.015 0.015	1.36 1.35	149.68 148.90	95 96	OK OK	OK OK
21		AM08	A005	A006	60	0.81	45.94	49	2788	4.07	11.63	47.35	9.19	0.00	144.34	375	1.05	0.015	1.41	155.73	93	OK	OK
22	Golf Links Road	AM08	A006	A007	60	3.26	49.20	196	2983	4.02	12.44	49.99	9.84	0.00	147.63	450	0.58	0.015	1.18	188.20	78	OK	OK
23 24		AM08 AL08	A007 A001	A001 A020	0 60	0.00 13.24	49.20 62.44	0 794	2983 3778	4.02 3.83	12.44 15.75	49.99 60.38	9.84 12.49	0.00	147.63 160.67	450 450	0.16 1.00	0.015 0.015	0.62 1.55	98.85 247.12	149 65	OK OK	UNDER OK
25		AL09	A020	A019	60	0.50	62.94	30	3808	3.83	15.88	60.76	12.59	0.00	161.15	525	0.21	0.015	0.79	170.82	94	OK	OK
26 27	" 	AL09	A019	A015	60	9.82	72.76	589	4397	3.72	18.34	68.18	14.55	0.00	170.53	525	0.34	0.015	1.00	217.36	78	OK OK	OK OK
28	Easement	AL09 AL09	A015 A014	A014 A048	60 0	9.93 1.34	82.69 84.03	596 0	4993 4993	3.62 3.62	20.82 20.82	75.47 75.47	16.54 16.81	0.00 0.00	179.81 180.08	600 600	0.92 0.19	0.013 0.013	2.08 0.95	589.02 267.68	31 67	OK	OK
29		AL08	A048	A049	23	5.34	89.37	123	5116	3.61	21.33	76.95	17.87	0.00	182.63	600	0.17	0.013	0.90	253.20	72	OK	OK
30 31	"	AL08	A049	A051	0	3.88	93.25	0	5116	3.61	21.33	76.95	18.65	0.00	183.40	600	0.07	0.013	0.57	162.47	113	LOW	UNDER
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Report III - Master Plan Class EA Report

Appendix A-2 (03)

Projects WW-06 and WW-07

Winona SPS (HC016) and Forcemain Upgrades



Design Flows to Winona SPS (HC016) – Full Buildout

		Existing	Ultimate
Nun	nber of Residential Units		
(A)	Entire Service Area	317	317
Oth	er Land Uses		
(B)	Commercial	4.80 ha	4.80 ha
(C)	Future Residential Development	O ha	21.57 ha
Des	ign Population		
(D)	[(A) * 3 pers/unit] + [(B) * 70 pers/ha] + [(C) * 100 pers/ha]	1,287	3,444
Des	ign Flow		
(E)	Average Flow (360 Lpcd)	5.4 L/s	14.4 L/s
(F)	Peak Factor, $M = 5/[(D/1000)^0.2]$	4.75	3.91
(G)	Domestic Peak Flow, (E) * (F)	25.7 L/s	56.3 L/s
	Developed Area	47.2 ha	68.8 ha
(H)	External Infiltration (at 0.2 L/ha/s)	9.4 L/s	13.8 L/s
тот	AL PEAK FLOW, (G) + (H)	35.1 L/s	70.1 L/s
Flov	v from Shippee-McCollum PS	60.0 L/s	80.0 L/s
Des	ign Flow for Winona PS	95.1 L/s	150.1 L/s

The existing capacity of the Winona PS forcemain is approximately 126 L/s







Report III - Master Plan Class EA Report

Appendix A-2 (04)

Projects WW-08, WW-09 and WW-10

Waterdown WWTP Decommissioning and SPS/Forcemain Construction



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed



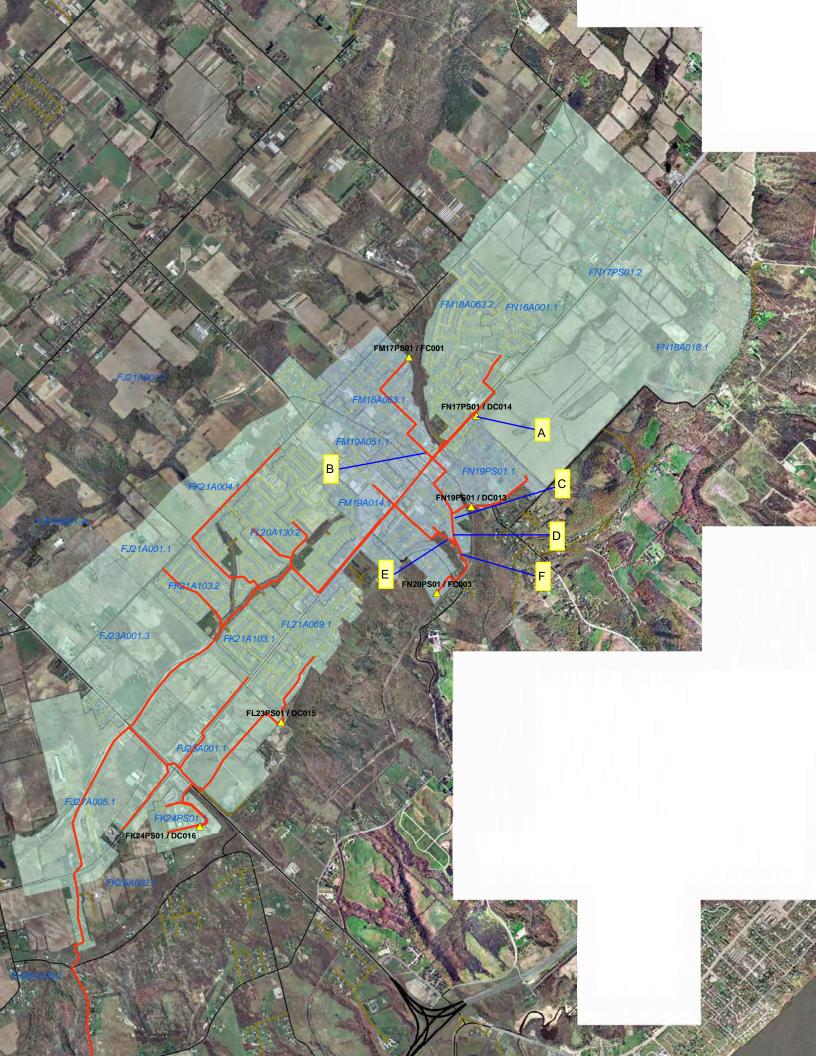


Wastewater Servicing

WW - 8,9,10,16 & 17 Forcemain & Gravity Sewer



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)1	Catchment	Status	Node/Manhole	Area	Pop	Pop Flow	Jobs	Jobs Flow	Avg. Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow	Trunk Flow Capacity
			Applied	[ha]		[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]	
	FN17PS01.1	Active	FN17PS01.1	12.93	248	0.9	5	0.0	0.9	5.0	4.4	2.6	7.0	0
	FN18A018.1	Undeveloped	FN18A018.1	139.38	9	0.0	30	0.1	0.1	5.0	0.6	27.9	28.5	207
	FN17PS01.2	Undeveloped	FN16A001.1	203.38	2115	7.3	433	1.3	8.6	4.1	35.9	40.7	76.5	104
	FM18A063.2	Active	FM18A063.1	85.39	1866	6.5	241	0.7	7.2	4.3	31.0	17.1	48.1	102
	FN16A001.1	Active	FN16A001.1	35.1	305	1.1	102	0.3	1.4	5.0	6.8	7.0	13.9	100
	(A) Flows Total			476.18	4543	15.8	812	2.4	18.2	3.6	65.1	95.2	160.3	400
	FM19A051.1	Active	FM19A051.1	35.52	642	2.2	214	0.6	2.9	5.0	14.4	7.1	21.5	148
	FM18A063.1	Active	FM18A063.1	37.44	1035	3.6	138	0.4	4.0	4.8	19.4	7.5	26.9	102
	FM17PS01.1	Active	FN17PS01.1	12.93	248	0.9	5	0.0	0.9	5.0	4.4	2.6	7.0	0
	(B) Flows Total			85.89	1926	6.7	357	1.1	7.8	4.2	32.9	17.2	50.1	123
	FN19PS01.1	Active	FN19PS01.1	29.95	538	1.9	33	0.1	2.0	5.0	9.9	6.0	15.8	282
	(C) Flows Total			115.84	2464	8.6	391	1.2	9.7	4.1	39.4	23.2	62.6	133
	FN19A005.1	Active	FN19A005.1	8.53	145	0.5	6	0.0	0.5	5.0	2.6	1.7	4.3	161
	(D) Flows Total			124.37	2609	9.1	397	1.2	10.3	4.0	41.1	30.9	72.0	
	FM19A007.1	Active	FM19A014.1	10.9	238	0.8	107	0.3	1.1	5.0	5.7	2.2	7.9	277
	FM19A014.1	Active	FM19A014.1	32.55	539	1.9	260	0.8	2.7	5.0	13.3	6.5	19.8	277
	E) Flows Total			43.45	777	2.7	368	1.1	3.8	4.9	18.5	8.7	27.2	277
	FM19A026.1	Active	FM19A026.1	15.59	297	1.0	28	0.1	1.1	5.0	5.6	3.1	8.7	472
	FN20PS01.1	Active	FM19A026.1	10.97	251	0.9	2	0.0	0.9	5.0	4.4	2.2	6.6	472
	(F) Flows Total			70.01	1324	4.6	397	1.2	5.8	4.5	26.0	14.0	40.0	472
	NEW PS totals			194.4	3,933.7	13.7	794.0	2.4	16.0	3.7	58.8	44.9	103.7	

11	Catchment	Status	Node/Manhole Applied	Area [ha]	Pop	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow	Peaking Factor	Peak Flow	Infiltration [L/s]	Total Flow	Truck Flow capacity
Ī	FN17PS01.1	Active	FN17PS01.1	12.93	256	0.9	6	0.0	0.9	5.0	4.5	2.6	7.1	0
	FN18A018.1	Undeveloped	FN18A018.1	139.38	8	0.0	31	0.1	0.1	5.0	0.6	27.9	28.5	207
	FN17PS01.2	Undeveloped	FN16A001.1	203.38	2201	7.6	484	1.5	9.1	4.1	37.3	40.7	78.0	104
	FM18A063.2	Active	FM18A063.1	85.39	1930	6.7	257	0.8	7.5	4.3	32.0	17.1	49.0	102
Ī	FN16A001.1	Active	FN16A001.1	35.1	316	1.1	123	0.4	1.5	5.0	7.3	7.0	14.4	100
	(A) Flows Total			476.18	4711	16.4	902	2.7	19.1	3.5	67.5	95.2	162.8	400
Π	FM19A051.1	Active	FM19A051.1	35.52	653	2.3	212	0.6	2.9	5.0	14.5	7.1	21.6	148
Ī	FM18A063.1	Active	FM18A063.1	37.44	1051	3.7	135	0.4	4.1	4.8	19.6	7.5	27.1	102
Ī	FM17PS01.1	Active	FN17PS01.1	12.93	256	0.9	6	0.0	0.9	5.0	4.5	2.6	7.1	0
Ī	(B) Flows Total			85.89	1961	6.8	354	1.1	7.9	4.2	33.3	17.2	50.5	123
Π	FN19PS01.1	Active	FN19PS01.1	29.95	549	1.9	35	0.1	2.0	5.0	10.1	6.0	16.1	282
	(C) Flows Total			115.84	2510	8.7	389	1.2	9.9	4.0	39.9	23.2	63.1	133
Ī	FN19A005.1	Active	FN19A005.1	8.53	148	0.5	6	0.0	0.5	5.0	2.7	1.7	4.4	161
	(D) Flows Total			124.37	2658	9.2	395	1.2	10.4	4.0	41.7	30.9	72.5	
Π	FM19A007.1	Active	FM19A014.1	10.9	238	0.8	107	0.3	1.1	5.0	5.7	2.2	7.9	277
	FM19A014.1	Active	FM19A014.1	32.55	539	1.9	260	0.8	2.7	5.0	13.3	6.5	19.8	277
	E) Flows Total			43.45	777	2.7	368	1.1	3.8	4.9	18.5	8.7	27.2	277
	FM19A026.1	Active	FM19A026.1	15.59	297	1.0	28	0.1	1.1	5.0	5.6	3.1	8.7	472
	FN20PS01.1	Active	FM19A026.1	10.97	251	0.9	2	0.0	0.9	5.0	4.4	2.2	6.6	472
	(F) Flows Total			70.01	1324	4.6	397	1.2	5.8	4.5	26.0	14.0	40.0	472
	NEW PS totals			194.4	3,982.2	13.8	792.8	2.4	16.2	3.7	59.3	44.9	104.2	

						Pop		Jobs		Peaking		Infiltration		Truck Flow
021	Catchment	Status	Node/Manhole	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Peak Flow		Total Flow	capacity
			Applied	[ha]		[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]	
	FN17PS01.1	Active	FN17PS01.1	12.93	360	1.3	7	0.0	1.3	5.0	6.4	2.6	9.0	0
	FN18A018.1	Undeveloped	FN18A018.1	139.38	8	0.0	957	2.9	2.9	5.0	14.5	27.9	42.4	207
	FN17PS01.2	Undeveloped	FN16A001.1	203.38	2645	9.2	1150	3.5	12.6	3.8	48.4	40.7	89.1	104
	FM18A063.2	Active	FM18A063.1	85.39	2306	8.0	277	0.8	8.8	4.1	36.6	17.1	53.6	102
	FN16A001.1	Active	FN16A001.1	35.1	445	1.5	143	0.4	2.0	5.0	9.9	7.0	16.9	100
	(A) Flows Total			476.18	5763	20.0	2534	7.6	27.6	3.3	90.5	95.2	185.7	400
	FM19A051.1	Active	FM19A051.1	35.52	654	2.3	242	0.7	3.0	5.0	15.0	7.1	22.1	148
	FM18A063.1	Active	FM18A063.1	37.44	1054	3.7	149	0.4	4.1	4.8	19.8	7.5	27.3	102
	FM17PS01.1	Active	FN17PS01.1	12.93	360	1.3	7	0.0	1.3	5.0	6.4	2.6	9.0	0
	(B) Flows Total			85.89	2069	7.2	398	1.2	8.4	4.2	35.0	17.2	52.2	123
	FN19PS01.1	Active	FN19PS01.1	29.95	554	1.9	38	0.1	2.0	5.0	10.2	6.0	16.2	282
	(C) Flows Total			115.84	2622	9.1	436	1.3	10.4	4.0	41.7	23.2	64.8	133
	FN19A005.1	Active	FN19A005.1	8.53	149	0.5	7	0.0	0.5	5.0	2.7	1.7	4.4	161
	(D) Flows Total			124.37	2771	9.6	443	1.3	11.0	4.0	43.4	30.9	74.2	
	FM19A007.1	Active	FM19A014.1	10.9	238	0.8	107	0.3	1.1	5.0	5.7	2.2	7.9	277
	FM19A014.1	Active	FM19A014.1	32.55	539	1.9	260	0.8	2.7	5.0	13.3	6.5	19.8	277
	E) Flows Total			43.45	777	2.7	368	1.1	3.8	4.9	18.5	8.7	27.2	277
	FM19A026.1	Active	FM19A026.1	15.59	297	1.0	28	0.1	1.1	5.0	5.6	3.1	8.7	472
	FN20PS01.1	Active	FM19A026.1	10.97	251	0.9	2	0.0	0.9	5.0	4.4	2.2	6.6	472
	(F) Flows Total			70.01	1324	4.6	397	1.2	5.8	4.5	26.0	14.0	40.0	472
	NEW PS totals			194.4	4,095.8	14.2	840.8	2.5	16.8	3.6	60.9	44.9	105.7	1

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Catchment	Status	Node/Manhole Applied	Area [ha]	Pop	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow [L/s]	Peaking Factor	Peak Flow [L/s]	Infiltration [L/s]	Total Flow [L/s]	Truck Flow capacity
FN17PS01.1	Active	FN17PS01.1	12.93	360	1.3	8	0.0	1.3	5.0	6.4	2.6	9.0	104
FN18A018.1	Undeveloped	FN18A018.1	139.38	3850	13.4	1021	3.1	16.4	3.6	59.9	27.9	87.8	207
FN17PS01.2	Undeveloped	FN16A001.1	203.38	8074	28.0	1226	3.7	31.7	3.2	101.5	40.7	142.2	104
FM18A063.2	Active	FM18A063.1	85.39	2306	8.0	296	0.9	8.9	4.1	36.7	17.1	53.8	102
FN16A001.1	Active	FN16A001.1	35.1	445	1.5	152	0.5	2.0	5.0	10.0	7.0	17.0	100
(A) Flows Total			476.18	15035	52.2	2703	8.1	60.3	2.8	169.7	95.2	265.0	400
FM19A051.1	Active	FM19A051.1	35.52	654	2.3	265	0.8	3.1	5.0	15.3	7.1	22.4	148
FM18A063.1	Active	FM18A063.1	37.44	1055	3.7	162	0.5	4.1	4.8	19.9	7.5	27.4	102
FM17PS01.1	Active	FM18A063.1	9.93	267	0.9	31	0.1	1.0	5.0	5.1	2.0	7.1	102
(B) Flows Total			82.89	1975	6.9	458	1.4	8.2	4.2	34.5	16.6	51.1	123
FN19PS01.1	Active	FN19PS01.1	29.95	554	1.9	41	0.1	2.0	5.0	10.2	6.0	16.2	282
(C) Flows Total			112.84	2529	8.8	499	1.5	10.3	4.0	41.2	22.6	63.8	133
FN19A005.1	Active	FN19A005.1	8.53	149	0.5	7	0.0	0.5	5.0	2.7	1.7	4.4	161
(D) Flows Total			121.37	2678	9.3	506	1.5	10.8	4.0	42.9	30.3	73.2	
FM19A007.1	Active	FM19A014.1	10.9	238	0.8	107	0.3	1.1	5.0	5.7	2.2	7.9	277
FM19A014.1	Active	FM19A014.1	32.55	539	1.9	260	0.8	2.7	5.0	13.3	6.5	19.8	277
E) Flows Total			43.45	777	2.7	368	1.1	3.8	4.9	18.5	8.7	27.2	277
FM19A026.1	Active	FM19A026.1	15.59	297	1.0	28	0.1	1.1	5.0	5.6	3.1	8.7	472
FN20PS01.1	Active	FM19A026.1	10.97	251	0.9	2	0.0	0.9	5.0	4.4	2.2	6.6	472
(F) Flows Total			70.01	1324	4.6	397	1.2	5.8	4.5	26.0	14.0	40.0	472
NEW PS totals			191.4	4,002.7	13.9	903.6	2.7	16.6	3.6	60.4	44.3	104.7	





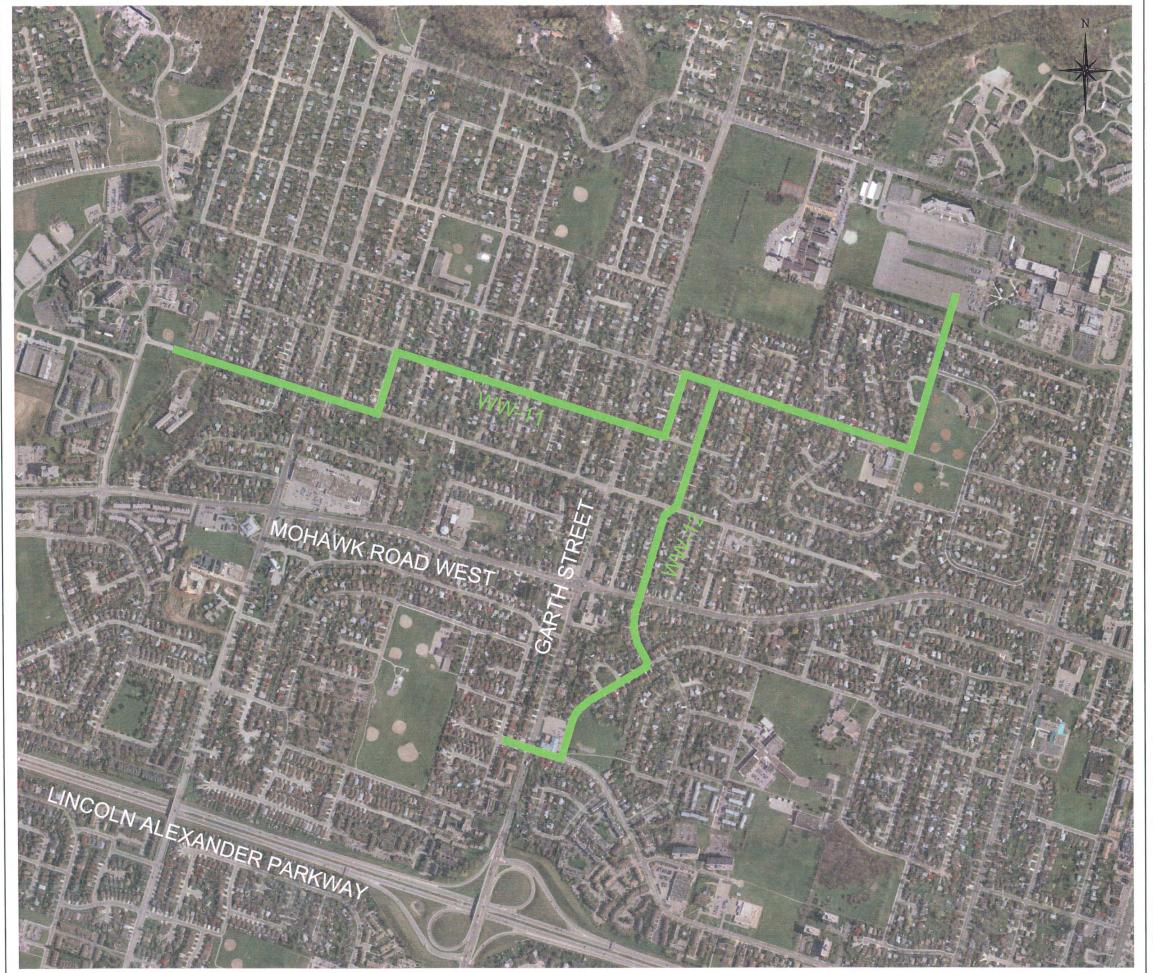


Report III - Master Plan Class EA Report

Appendix A-2 (05)

Project WW-11

Ancaster-to-Fennell Trunk Sewer Upgrades



Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases



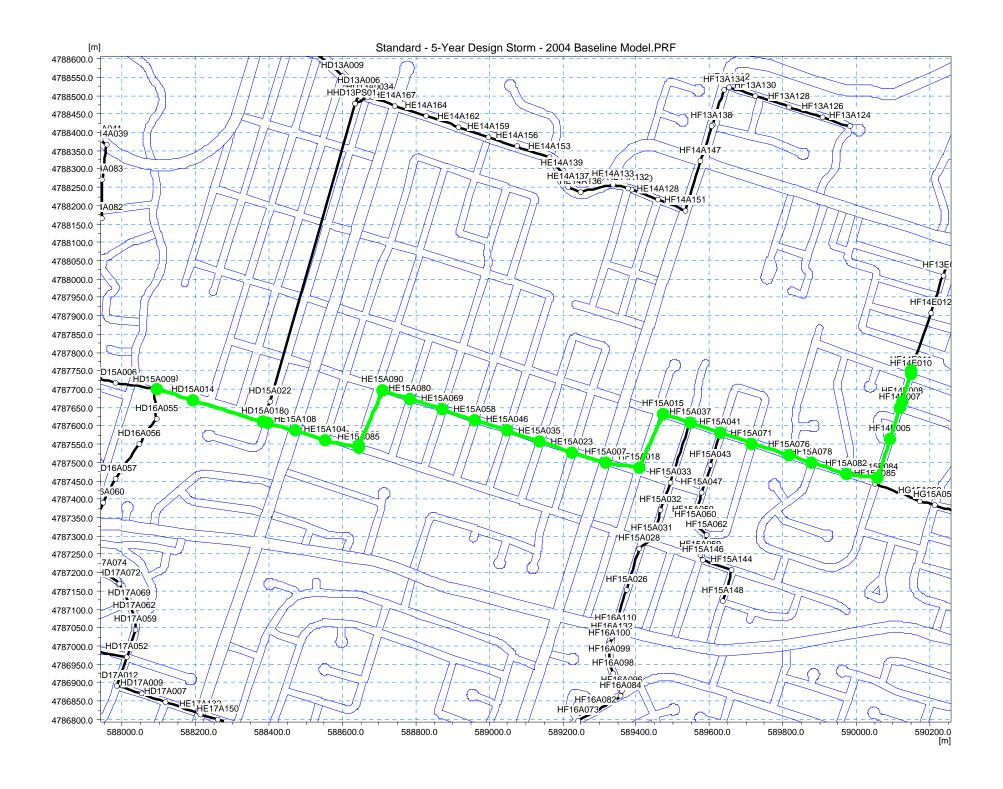


Wastewater Servicing

WW - 11 & 12 Gravity Sewer



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Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2004 Baseline Model.PRF

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193.5 -															 				_	_ -		
193.0 -								 		- 					 	 -		<u> </u>			 	
	0.0	200.0	400	.0	600.0	800.0	0	1000.0	1:	200.0	1400	.0	1600.0	18	800.0	2000	0.0	2200.0	240	0.00	2600.0 	[m]
Ground Lev.		199.30	201.23	202.60	202.58	198.30	199.18	199.44	200.98	200.69	198.83	198.01		198.33	198.53	198.09	198.58 198.93	198.35	198.88	199.49	199.03	[m]
Invert lev.		195.54	195.29		194.94	194.50		27	194.20	က	194.09			193.84	193.76	193.68	193.66			193.18	193.02	[m]
Length	108.51		90.07	86.11 103					3.65 92.	66 94.03		103.02	169.22		7.33 90.4			96.62 97.8			32.30	[m]
Diameter	0.90	0.90	1.05	1.05 1.0	05 1.0	5 1.2	20 1.20	1.20	1.20 1.2	20 1.20	1.20	1.20	1.20	1.20 1	.20 1.20	1.20	1.20	1.20 1.20	0 1.35	1.35	1.35	[m]
Slope o/oo	1.29	1.29	1.67	0.35 1.0	06 2.4	6 1.7	9 0.32	0.64	0.75 0.7	76 0.21	0.22	1.36	0.71	0.52	0.88	0.19	1.35	0.31 1.64	4 0.82	0.88	0.81	





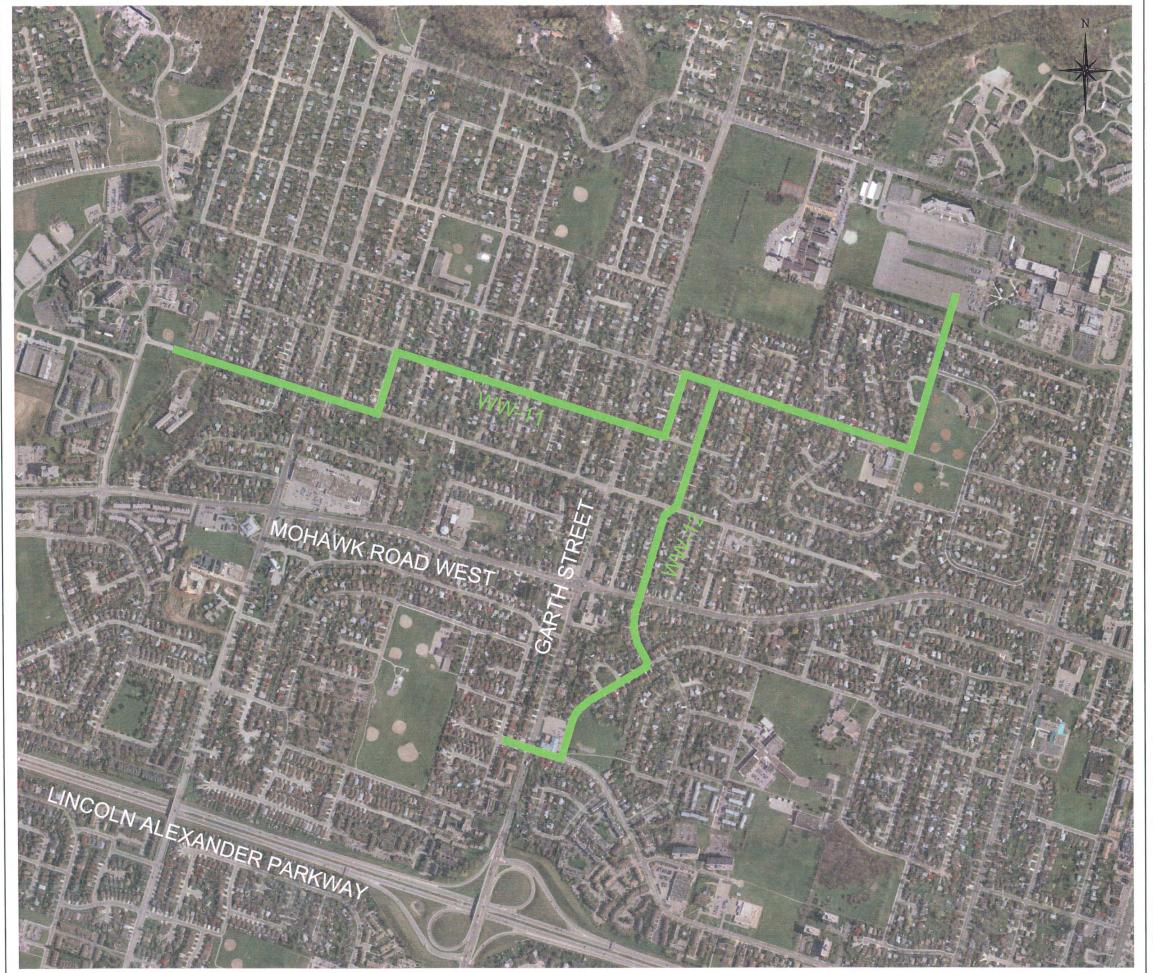


Report III - Master Plan Class EA Report

Appendix A-2 (06)

Project WW-12

West 18th St. Sewer Upgrades



Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases



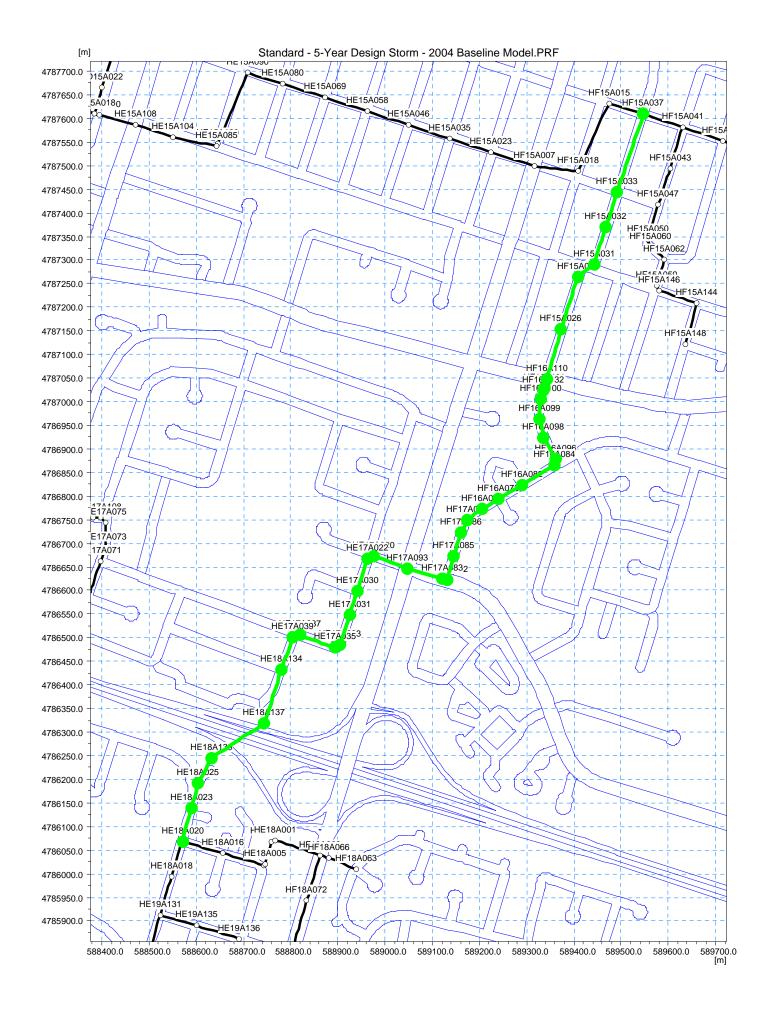


Wastewater Servicing

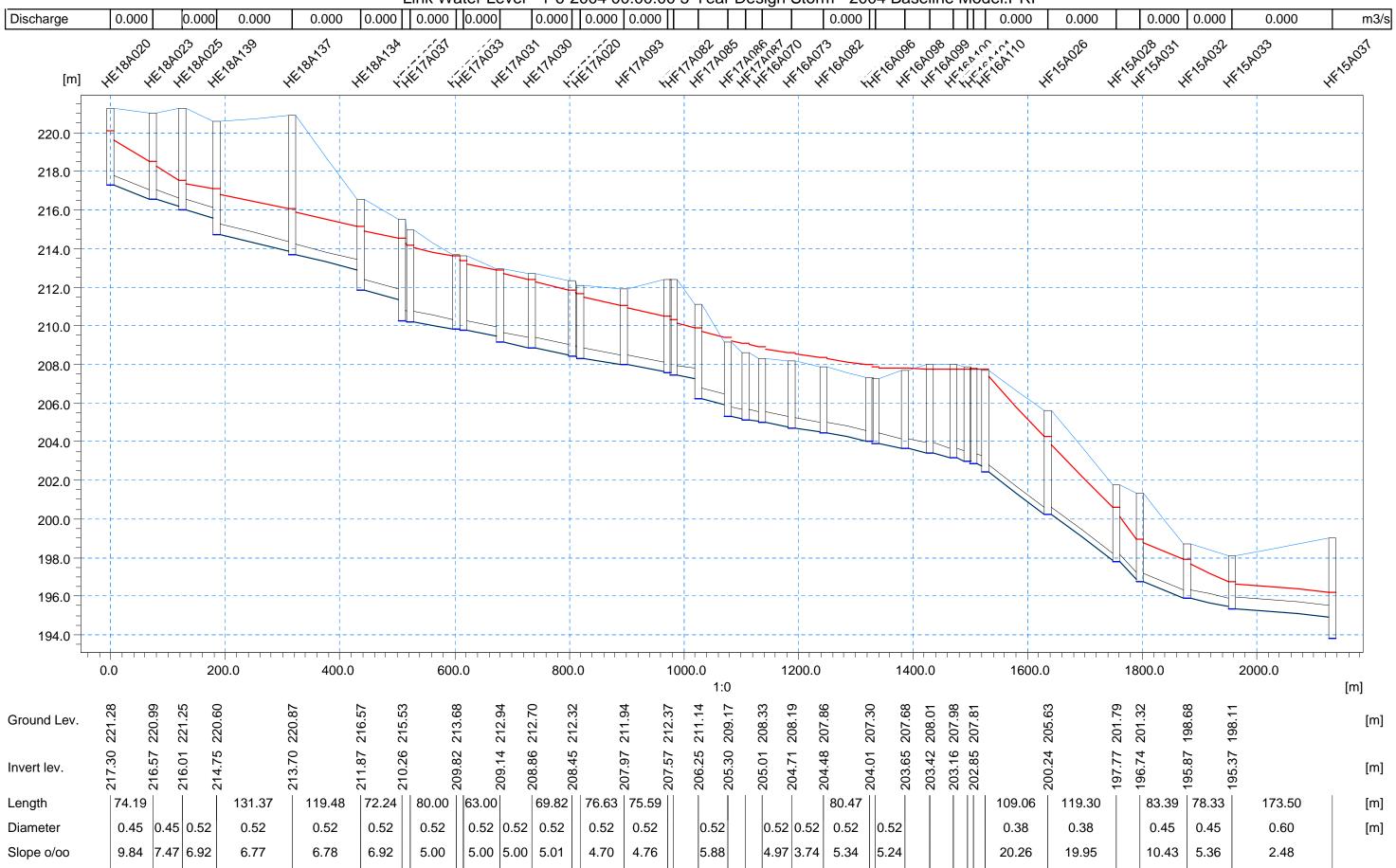
WW - 11 & 12 Gravity Sewer



22 NOV 06 1:10,000 2590-D-89



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2004 Baseline Model.PRF









Report III - Master Plan Class EA Report

Appendix A-2 (07)

Project WW-13

Trunk CSO and Collection System Improvements







Report III - Master Plan Class EA Report

Appendix A-2 (08)

Project WW-14

New Centennial Trunk Sewer



Vote:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.



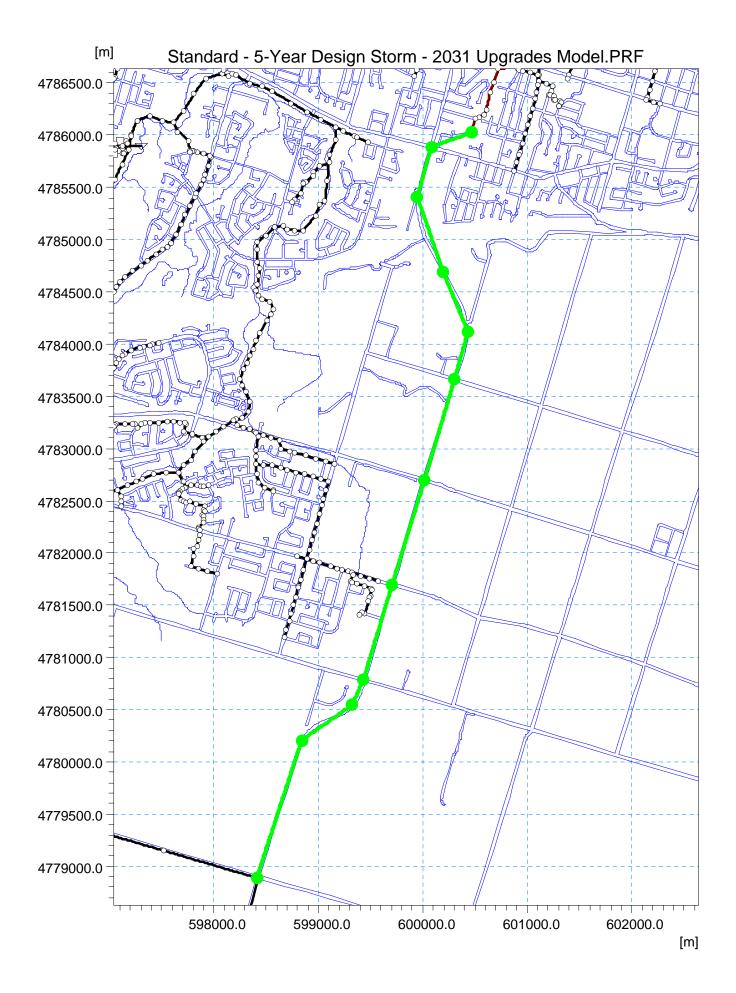


Wastewater Servicing

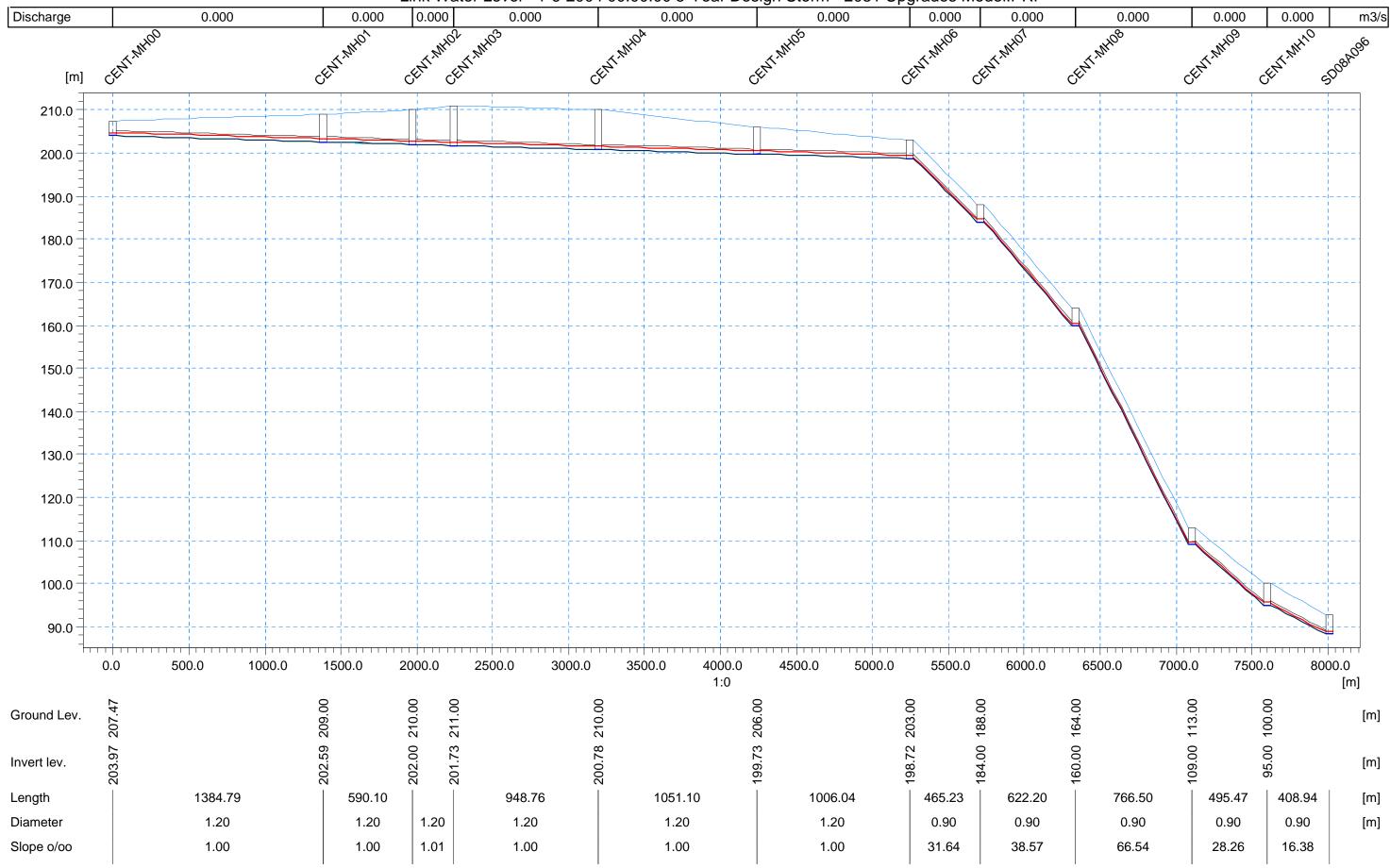
WW-14 Gravity Sewer



22 NOV 06 1:30,000 2590-D-85



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2031 Upgrades Model.PRF







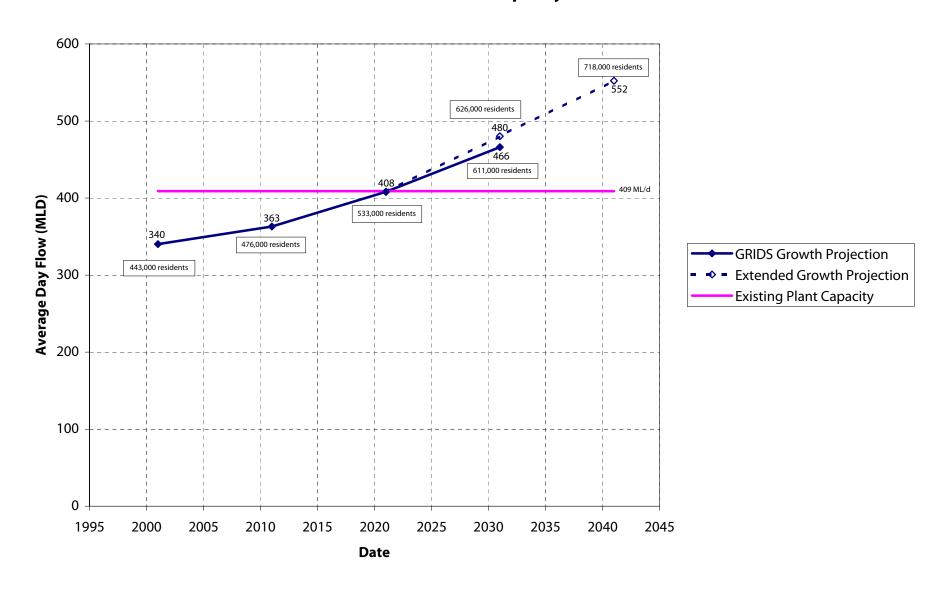


Report III - Master Plan Class EA Report

Appendix A-2 (09)

Project WW-15
Woodward Avenue WWTP Expansion

Woodward Avenue WWTP Capacity Assessment









Report III - Master Plan Class EA Report

Appendix A-2 (10)

Project WW-16

New Mountain Brow Trunk Sewer



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed



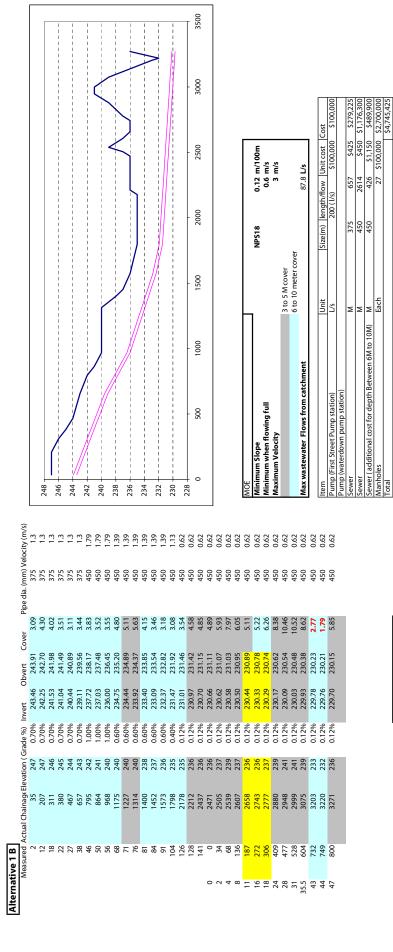


Wastewater Servicing

WW - 8,9,10,16 & 17 Forcemain & Gravity Sewer



22 NOV 06 1:12,500 2590-D-87



M Each







Report III - Master Plan Class EA Report

Appendix A-2 (11)

Project WW-17

First Street SPS (DC014) Upgrades

LOCATION CATCHMENTID FN16A001 FN16A001.1 FN17PS01 FN17PS01.1 FN17PS01.2

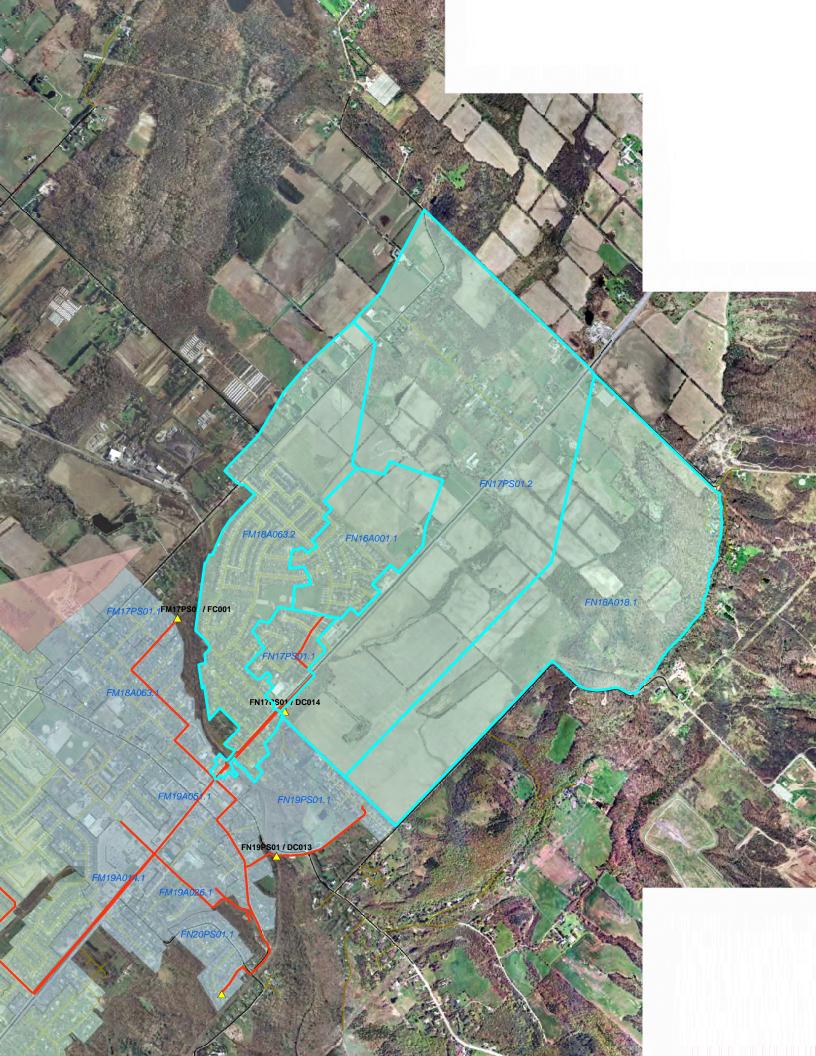
Highway No.5/First Street
Station ID: DC014
Model ID: FN17PS01
Pumps: 2 @ 200 L/s each
Firm Capacity: 200 L/s

2001	Catchment	Status	Area	Pop	Pop Flow	Jobs	Jobs Flow	Avg. Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow
2001	Catchinent	Status	[ha]	гор	[L/s]	Jobs	[L/s]	[L/s]	ractor	[L/s]	[L/s]	[L/s]
	FN16A001.1	Active	35.1	305	1.1	102	0.3	1.4	3.6	4.9	7.0	11.9
	FM18A063.2	Active	85.39	1866	6.5	241	0.7	7.2	3.6	25.8	17.1	42.8
	FN17PS01.1	Active	12.93	248	0.9	5	0.0	0.9	3.6	3.1	2.6	5.7
	FN17PS01.2	Undeveloped	203.38	2115	7.3	433	1.3	8.6	3.6	30.9	40.7	71.6
	FN18A018.1	Undeveloped	139.38	9	0.0	30	0.1	0.1	3.6	0.4	27.9	28.3
	TOTAL	•	476.18	4543	16	812	2	18	3.6	65.1	95	160.3
					D		laba		Daaldaa	Dool.		Tatal
2011	Catchment	Ctatus		D	Pop Flow	laba	Jobs Flow	Ava Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow
2011	Catchment	Status	Area	Pop		Jobs		Avg. Flow	Factor		FI (=1	
	FN16A001.1	Active	[ha] 35.1	316	[L/s] 1.1	123	[L/s] 0.4	[L/s] 1.5	3.5	[L/s] 5.2	[L/s] 7.0	[L/s] 12.2
	FM18A063.2	Active	85.39	1930	6.7	257	0.4	7.5	3.5 3.5	5.∠ 26.5	7.0 17.1	43.5
	FN17PS01.1	Active	12.93	256	0.7	6	0.0	0.9	3.5	3.2	2.6	5.8
	FN17PS01.1	Undeveloped	203.38	2201	7.6	484	1.5	9.1	3.5	32.2	40.7	72.9
	FN18A018.1	Undeveloped	139.38	8	0.0	31	0.1	0.1	3.5	0.4	27.9	28.3
	TOTAL	Ondeveloped	476.18	4711	16.4	902	2.7	19.1	3.5	67.5	95.2	162.8
	TOTAL		410.10	4,	10.4	302		10.1	0.0	01.0	30.2	102.0
					Pop		Jobs		Peaking	Peak		Total
2021	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Flow	Infiltration	Flow
	Gatolillicit	Otatas		. ор	[L/s]	0000		[L/s]	· uoto.	[L/s]	[L/s]	[L/s]
							[] /e]					
	FN16A001 1	Active	[ha] 35.1	445		143	[L/s]		3.3			
	FN16A001.1 FM18A063.2	Active Active	35.1	445 2306	1.5	143 277	0.4	2.0	3.3	6.5	7.0	13.5
	FM18A063.2	Active	35.1 85.39	2306	1.5 8.0	277	0.4 0.8	2.0 8.8	3.3	6.5 29.0	7.0 17.1	13.5 46.0
	FM18A063.2 FN17PS01.1	Active Active	35.1 85.39 12.93		1.5 8.0 1.3	277 7	0.4 0.8 0.0	2.0 8.8 1.3	3.3 3.3	6.5 29.0 4.2	7.0 17.1 2.6	13.5 46.0 6.8
	FM18A063.2	Active	35.1 85.39	2306 360	1.5 8.0	277	0.4 0.8	2.0 8.8	3.3	6.5 29.0	7.0 17.1	13.5 46.0
	FM18A063.2 FN17PS01.1 FN17PS01.2	Active Active Undeveloped	35.1 85.39 12.93 203.38	2306 360 2645	1.5 8.0 1.3 9.2	277 7 1150	0.4 0.8 0.0 3.5	2.0 8.8 1.3 12.6	3.3 3.3 3.3	6.5 29.0 4.2 41.4	7.0 17.1 2.6 40.7	13.5 46.0 6.8 82.1
	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1	Active Active Undeveloped	35.1 85.39 12.93 203.38 139.38	2306 360 2645 8	1.5 8.0 1.3 9.2 0.0	277 7 1150 957	0.4 0.8 0.0 3.5 2.9	2.0 8.8 1.3 12.6 2.9	3.3 3.3 3.3 3.3	6.5 29.0 4.2 41.4 9.5	7.0 17.1 2.6 40.7 27.9	13.5 46.0 6.8 82.1 37.4
	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1	Active Active Undeveloped	35.1 85.39 12.93 203.38 139.38	2306 360 2645 8	1.5 8.0 1.3 9.2 0.0 20.0	277 7 1150 957	0.4 0.8 0.0 3.5 2.9 7.6	2.0 8.8 1.3 12.6 2.9	3.3 3.3 3.3 3.3 3.3	6.5 29.0 4.2 41.4 9.5 90.5	7.0 17.1 2.6 40.7 27.9	13.5 46.0 6.8 82.1 37.4 185.7
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL	Active Active Undeveloped Undeveloped	35.1 85.39 12.93 203.38 139.38 476.18	2306 360 2645 8 5763	1.5 8.0 1.3 9.2 0.0 20.0	277 7 1150 957 2534	0.4 0.8 0.0 3.5 2.9 7.6	2.0 8.8 1.3 12.6 2.9 27.6	3.3 3.3 3.3 3.3 3.3	6.5 29.0 4.2 41.4 9.5 90.5	7.0 17.1 2.6 40.7 27.9	13.5 46.0 6.8 82.1 37.4 185.7
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1	Active Active Undeveloped	35.1 85.39 12.93 203.38 139.38 476.18	2306 360 2645 8	1.5 8.0 1.3 9.2 0.0 20.0	277 7 1150 957	0.4 0.8 0.0 3.5 2.9 7.6	2.0 8.8 1.3 12.6 2.9 27.6	3.3 3.3 3.3 3.3 3.3	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow	7.0 17.1 2.6 40.7 27.9 95.2	13.5 46.0 6.8 82.1 37.4 185.7
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL	Active Active Undeveloped Undeveloped	35.1 85.39 12.93 203.38 139.38 476.18	2306 360 2645 8 5763	1.5 8.0 1.3 9.2 0.0 20.0	277 7 1150 957 2534 Jobs	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s]	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s]	3.3 3.3 3.3 3.3 3.3 Peaking	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s]	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [L/s]	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s]
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL Catchment FN16A001.1	Active Active Undeveloped Undeveloped Status Active	35.1 85.39 12.93 203.38 139.38 476.18 Area [ha] 35.1	2306 360 2645 8 5763	1.5 8.0 1.3 9.2 0.0 20.0 Pop Flow [L/s] 1.5	277 7 1150 957 2534 Jobs	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s] 0.5	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s] 2.0	3.3 3.3 3.3 3.3 3.3 Peaking Factor	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s] 5.6	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [L/s]	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s] 12.7
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL Catchment FN16A001.1 FM18A063.2	Active Active Undeveloped Undeveloped Status Active Active	35.1 85.39 12.93 203.38 139.38 476.18 Area [ha] 35.1 85.39	2306 360 2645 8 5763 Pop	1.5 8.0 1.3 9.2 0.0 20.0 Pop Flow [L/s] 1.5 8.0	277 7 1150 957 2534 Jobs	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s] 0.5 0.8	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s] 2.0 8.8	3.3 3.3 3.3 3.3 3.3 Peaking Factor	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s] 5.6 29.0	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [L/s] 7.0 17.1	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s] 12.7 46.0
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL Catchment FN16A001.1 FM18A063.2 FN17PS01.1	Active Active Undeveloped Undeveloped Status Active Active Active	35.1 85.39 12.93 203.38 139.38 476.18 Area [ha] 35.1 85.39 12.93	2306 360 2645 8 5763 Pop 445 2306 360	1.5 8.0 1.3 9.2 0.0 20.0 Pop Flow [L/s] 1.5 8.0 1.3	277 7 1150 957 2534 Jobs 152 277 7	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s] 0.5 0.8 0.0	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s] 2.0 8.8 1.3	3.3 3.3 3.3 3.3 3.3 Peaking Factor	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s] 5.6 29.0 4.2	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [Us] 7.0 17.1 2.6	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s] 12.7 46.0 6.8
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL Catchment FN16A001.1 FM18A063.2 FN17PS01.1 FN17PS01.1	Active Active Undeveloped Undeveloped Status Active Active Active Undeveloped	35.1 85.39 12.93 203.38 139.38 476.18 Area [ha] 35.1 85.39 12.93 203.38	2306 360 2645 8 5763 Pop 445 2306 360 8074	1.5 8.0 1.3 9.2 0.0 20.0 Pop Flow [L/s] 1.5 8.0 1.3 28.0	277 7 1150 957 2534 Jobs 152 277 7	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s] 0.5 0.8 0.0 3.7	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s] 2.0 8.8 1.3 31.7	3.3 3.3 3.3 3.3 3.3 Peaking Factor 2.8 3.3 3.3 2.8	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s] 5.6 29.0 4.2 89.3	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [L/s] 7.0 17.1 2.6 40.7	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s] 12.7 46.0 6.8 129.9
2031	FM18A063.2 FN17PS01.1 FN17PS01.2 FN18A018.1 TOTAL Catchment FN16A001.1 FM18A063.2 FN17PS01.1	Active Active Undeveloped Undeveloped Status Active Active Active	35.1 85.39 12.93 203.38 139.38 476.18 Area [ha] 35.1 85.39 12.93	2306 360 2645 8 5763 Pop 445 2306 360	1.5 8.0 1.3 9.2 0.0 20.0 Pop Flow [L/s] 1.5 8.0 1.3	277 7 1150 957 2534 Jobs 152 277 7	0.4 0.8 0.0 3.5 2.9 7.6 Jobs Flow [L/s] 0.5 0.8 0.0	2.0 8.8 1.3 12.6 2.9 27.6 Avg. Flow [L/s] 2.0 8.8 1.3	3.3 3.3 3.3 3.3 3.3 Peaking Factor	6.5 29.0 4.2 41.4 9.5 90.5 Peak Flow [L/s] 5.6 29.0 4.2	7.0 17.1 2.6 40.7 27.9 95.2 Infiltration [Us] 7.0 17.1 2.6	13.5 46.0 6.8 82.1 37.4 185.7 Total Flow [L/s] 12.7 46.0 6.8

Highway No.5/First Street

Pumping Station Attributes
Station ID: DC014
Model ID: FN17PS01
Pumps: 2 @ 200 L/s each
Firm Capacity: 200 L/s

i iiii Capacity. 200 L/3				
	2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Population Projections				
(A) Total Serviced Population	4543	4711	5763	15035
Employment Projections				
(B) Number of Jobs	812	812 902 2		2684
Design Flow				
(C) Area	476.2 ha	476.2 ha	476.2 ha	476.2 ha
(D) Average Residential Flow (300 Lpcd)	15.8 L/s	16.4 L/s	20.0 L/s	52.2 L/s
(E) Average Employment Flow (260 Lpcd)	2.4 L/s	2.7 L/s	7.6 L/s	8.1 L/s
(F) Average Flow	18.2 L/s	19.1 L/s	27.6 L/s	60.3 L/s
(G) Peaking Factor	3.6	3.5	3.3	2.8
(H) Domestic Peak Flow, (G)*(F)	65.1 L/s	67.5 L/s	90.5 L/s	169.6 L/s
(I) External Infiltration, (C)*0.2 L/ha/s	95.2 L/s	95.2 L/s	95.2 L/s	95.2 L/s
TOTAL PEAK FLOW. (H) + (I)	160.3 L/s	162.8 L/s	185.7 L/s	269.5 L/s









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Appendix A-2 (12)

Project WW-18
Scenic Drive Sewer Upgrades



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2004 Baseline Model.PRF

Discharge		0.000	0.000	0.0		000	0.000	0.000	0.000		0.000	0.000	0.000	0.000	m3/s
	HDA	, _k 061	hiD ander	HO14AO6	, HD14A060	HCJAA	\$1 ²³	C14A025	HC14A021	HC1AAC	5 ²⁸	HC194030	, post	HC14A033	,1C,32060
192.0															
190.0								-						_	
-															
188.0	 														
186.0			+	 											
184.0	<u>i</u>									<u>i</u>					
182.0													i 		
180.0															
178.0													 		
-															
176.0	 		 						 	 			1		
174.0			 	 		 	 			 			+		
172.0	<u> </u>					<u> </u>				<u>1</u>			<u> </u>		
170.0													 		\
168.0													¦		
166.0			 			+				 					+
-	 		 						 	 					
164.0	 		 							 					
162.0						 				-					
160.0				<u> </u>		 			-	 	-				
	0.0		50.0	100.0)	150.0	200.0		250.0 1:0	300.0	350.0	40	00.0 450.0	500	
Ground Lev.	192.50		192.28	191.15	190.49	189.83	9	189.03 3.03	189.96	190.16		190.58		190.38	[m]
Invert lev.	188.06		187.84	186.84	186.78	186.70			186.54	186.42		186.29		166.77	[m]
Length		45.72	45.72	29.		.93	45.42	37.49	61.26		62.18	25.60	76.81	44.80	[m]
Diameter		0.38	0.38	0.3	38 0.	38	0.38	0.38	0.38		0.38	0.38	0.38	0.38	[m]
Slope o/oo		4.81	5.03	2.0	01 2.	00	1.98	1.87	1.96		2.09	2.73	1.69	134.15	







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Appendix A-2 (13)

Project WW-19

Bowman Street Sewer Upgrades



Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases





Wastewater Servicing

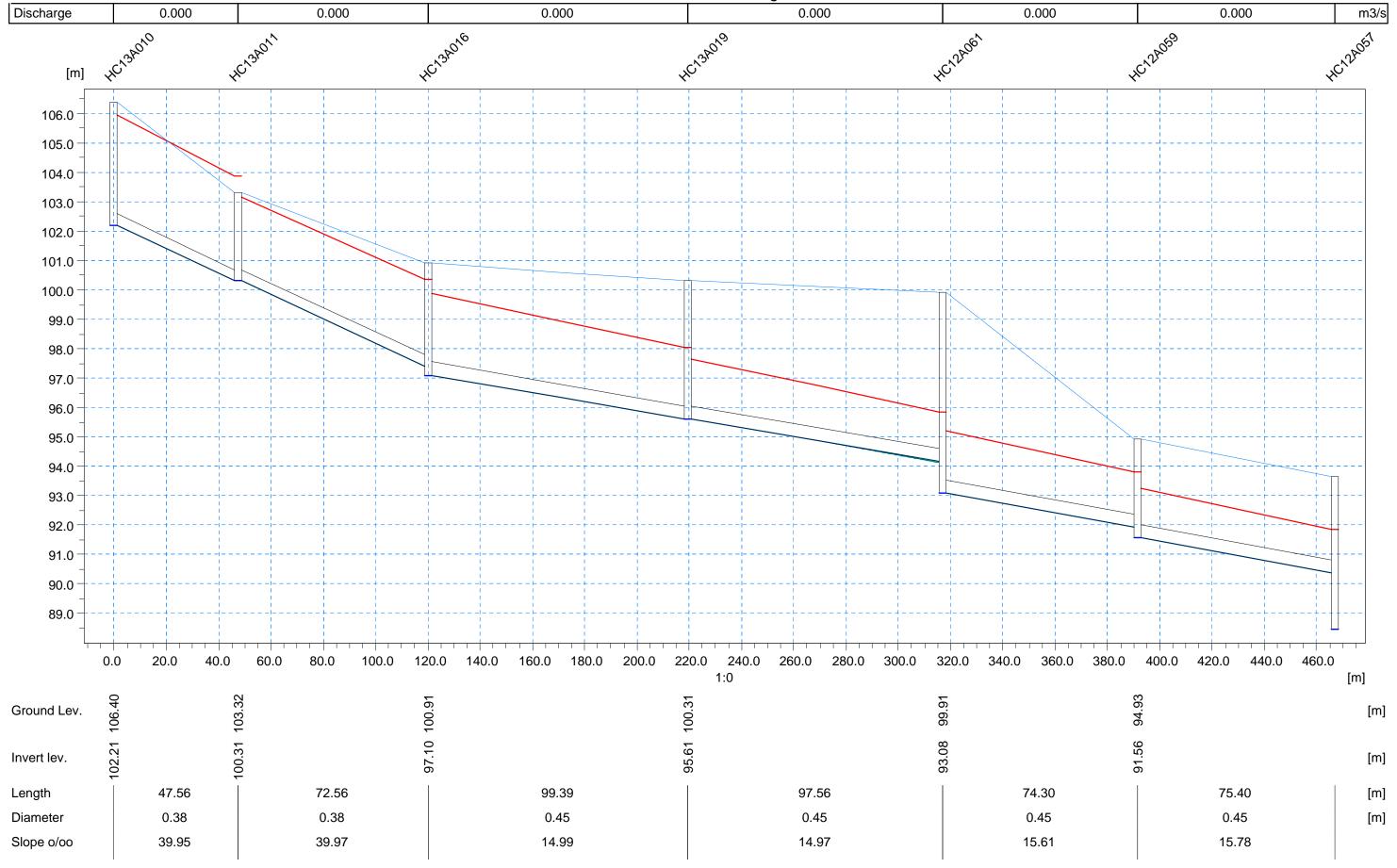
WW - 19 & 22 Forcemain & Gravity Sewer



22 NOV 06 1:10,000 2590-D-88



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2004 Baseline Model.PRF









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Appendix A-2 (14)

Projects WW-20 and WW-21

Binbrook SPS (HC058) and Forcemain Upgrades

Binbrook SPS

Station ID: HC058 Model ID: GP14PS01 Pumps: 2 @ 200 L/s each Firm Capacity: 200 L/s

2001	Catchment GS21A003.1 TOTAL GS21PS01 TOTAL	Status Active	Area [ha] 630 630	Pop 2000 2000	Pop Flow [L/s] 6.9 6.9	Jobs 301 301	Jobs Flow [L/s] 0.9 0.9	Avg. Flow [L/s] 7.9 7.9	Peaking Factor 4.2 4.2	Peak Flow [L/s] 33.2 33.2	Infiltration [L/s] 126.0 126.0	Total Flow [L/s] 159.2 159.2 30.0 189.2
2011	Catchment	Status	Area [ha]	Pop	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow [L/s]	Peaking Factor	Peak Flow [L/s]	Infiltration [L/s]	Total Flow [L/s]
	GS21A003.1 TOTAL GS21PS01 TOTAL	Active	630 630	5408 5408	18.8 18.8	823 823	2.5 2.5	21.3 21.3	4.2 3.5	90.0 73.7	126.0 126.0	216.0 216.0 30.0 246.0
					Pop		Jobs	Avg.	Peaking	Peak	Infiltration	Total
2021	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Flow	Factor	Flow		Flow
2021	Catchment GS21A003.1 TOTAL GS21PS01 TOTAL	Status Active	Area [ha] 630 630	Pop 9957 9957	Flow [L/s] 34.6 34.6	Jobs 1531 1531	Flow [L/s] 4.6 4.6	Flow [L/s] 39.2 39.2	4.2 3.1	Flow [L/s] 165.8 120.2	[L/s] 126.0 126.0	Flow [L/s] 291.8 291.8 30.0 321.8
2021	GS21A003.1 TOTAL GS21PS01		[ha] 630	9957	[L/s] 34.6	1531	[L/s] 4.6	[L/s] 39.2	4.2	[L/s] 165.8	[L/s] 126.0	[L/s] 291.8 291.8 30.0

Binbrook SPS

Pumping Station Attributes
Station ID: HC058
Model ID: GP14PS01
Pumps: 2 @ 200 L/s each
Firm Capacity: 200 L/s

	Firm Capacity. 200 L/S				
		2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Pop	oulation Projections				
(A)	Total Serviced Population	2000	5408	9957	14425
Em	ployment Projections				
(B)	Number of Jobs	301	823	1531	2382
Des	ign Flow				
(C)	Area	630.0 ha	630.0 ha	630.0 ha	630.0 ha
(D)	Average Residential Flow (300 Lpcd)	6.9 L/s	18.8 L/s	34.6 L/s	50.1 L/s
(E)	Average Employment Flow (260 Lpcd)	0.9 L/s	2.5 L/s	4.6 L/s	7.2 L/s
(F)	Average Flow	7.9 L/s	21.3 L/s	39.2 L/s	57.3 L/s
(G)	Peaking Factor	4.2	3.5	3.1	2.8
(H)	Domestic Peak Flow, (G)*(F)	33.2 L/s	73.7 L/s	120.2 L/s	162.8 L/s
(I)	External Infiltration, (C)*0.2 L/ha/s	126.0 L/s	126.0 L/s	126.0 L/s	126.0 L/s
TOT	TAL PEAK FLOW, (H) + (I)	189.2 L/s	246.0 L/s	321.8 L/s	398.3 L/s







Report III - Master Plan Class EA Report

Appendix A-2 (15)

Project WW-22

Highway 403 Trunk Sewer Upgrades



Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases



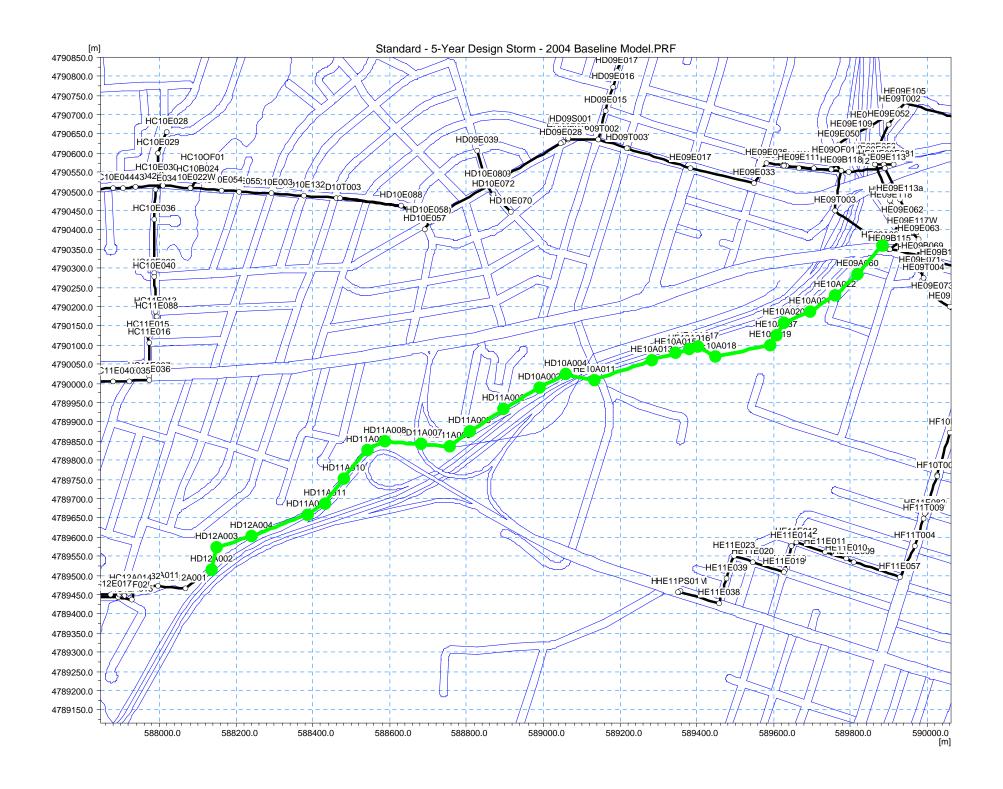


Wastewater Servicing

WW - 19 & 22 Forcemain & Gravity Sewer



22 NOV 06 1:10,000 2590-D-88



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2004 Baseline Model.PRF

Discharge	0.000 0.00	000 0.000 0.000 0.000 0.000 0.000 m3/s
	HOLYBOOK HOLYBOOK HOLYBOLY HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK HOLYBOOK	HE JOHO 13 OF OLIVE OF THE OPOLO HE JOHO OF THE OPOLO HE
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81.5		
٦	0.0 100.0 200.0 300.0 400.0 500.0 600.0 700.0 800.0 900.0 1000.0 1100.0 1200.0 1:0	1300.0 1400.0 1500.0 1600.0 1700.0 1800.0 1900.0 2000.0 2100.0 [m]
Ground Lev.	89.00 89.42 86.67 86.96 86.84 86.86 86 86 86 86 86 86 86 86 86 86 86 86 8	85.34 88.51 86.91 86.91 85.10 87.27
Invert lev.	88	[m] 47 68 82 11 47 68 68 68 68 68 68 68 68 68 68 68 68 68
Length		8.00 64.60 146.80 74.30 77.40 81.90 97.30 [m]
Diameter		0.90 0.
Slope o/oo	1.85 2.43 1.46 1.95 2.33 2.03 2.01 1.84 1.23 1.99 1.65 2.33 2.35 1	.14 2.79 2.71 1.70 2.02 1.81 2.56 1.85







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Appendix A-2 (16)

Project WW-23

New Airport Lands SPS

New Airport Lands SPS Station ID: Model ID: Pumps: Firm Capacity:

2001	Catchment	Status	Area	Pop	Pop Flow	Jobs	Jobs Flow	Avg. Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow
			[ha]		[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	TZ-2645	Undeveloped	514	0	0.0	811	2.4	2.4	4.0	9.7	102.8	112.5
	TZ-2649	Undeveloped	1170	0	0.0	2301	6.9	6.9	4.0	27.6	234.0	261.6
	TOTAL		1684.00	0	0.0	3112	9.4	9.4	4.0	37.3	336.8	374.1
2044		-		_	Pop		Jobs	A 51	Peaking	Peak	Infiltration	Total
2011	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Flow		Flow
	T7 0045		[ha]		[L/s]	000	[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	TZ-2645	Undeveloped	514	0	0.0	938	2.8	2.8	3.8	10.8	102.8	113.6
	TZ-2649	Undeveloped	1170	0	0.0	2818	8.5	8.5	3.8	32.5	234.0	266.5
	TOTAL		1684.00	0	0.0	3756	11.3	11.3	3.8	43.4	336.8	380.2
					Pop		Jobs		Peaking	Peak		Total
2021	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Flow	Infiltration	Flow
			[ha]	•	[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	TZ-2645	Undeveloped	514	0	0.0	2520	7.6	7.6	3.3	25.3	102.8	128.1
	TZ-2649	Undeveloped	1170	0	0.0	5078	15.3	15.3	3.3	50.9	234.0	284.9
	TOTAL	·	1684.00	0	0.0	7598	22.9	22.9	3.3	76.2	336.8	413.0
					Pop		Jobs		Peaking	Peak		Total
2031	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Flow	Infiltration	Flow
_			[ha]	•	[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	TZ-2645	Undeveloped	514	0	0.0	7168	21.6	21.6	2.8	60.4	102.8	163.2
	TZ-2649	Undeveloped	1170	0	0.0	10932	32.9	32.9	2.8	92.2	234.0	326.2
	TOTAL		1684.00	0	0.0	18100	54.5	54.5	2.8	152.6	336.8	489.4







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Appendix A-2 (17)

Projects WW-24 and WW-25

New Highway 6 Forcemain and Trunk Sewer



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed



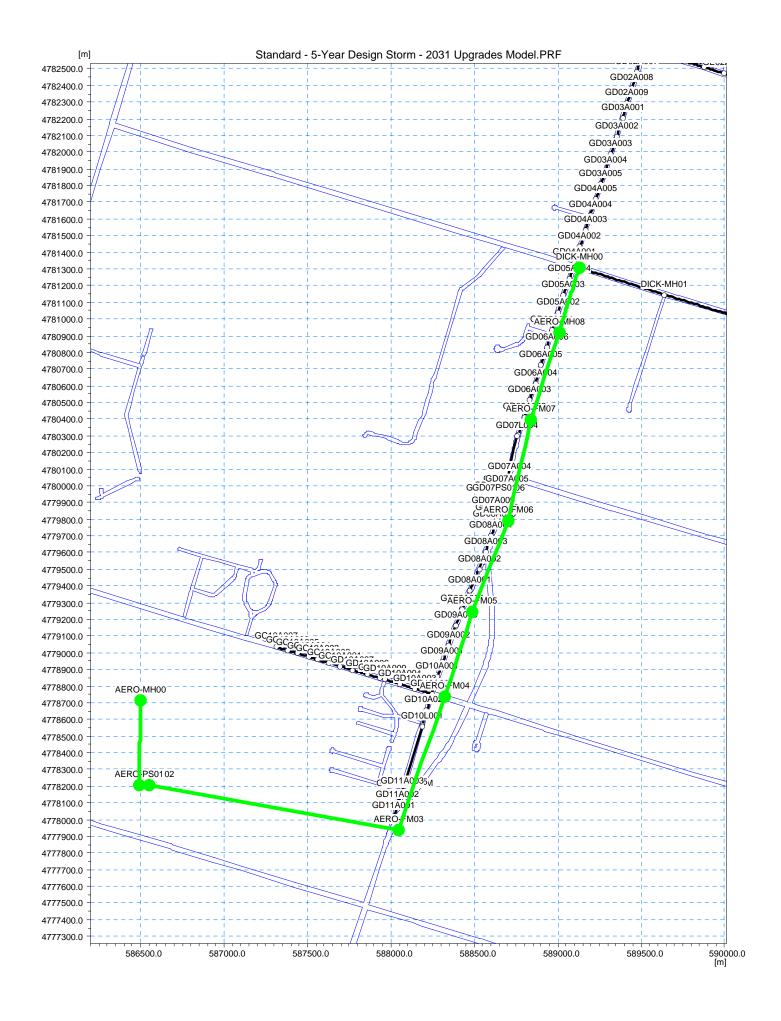


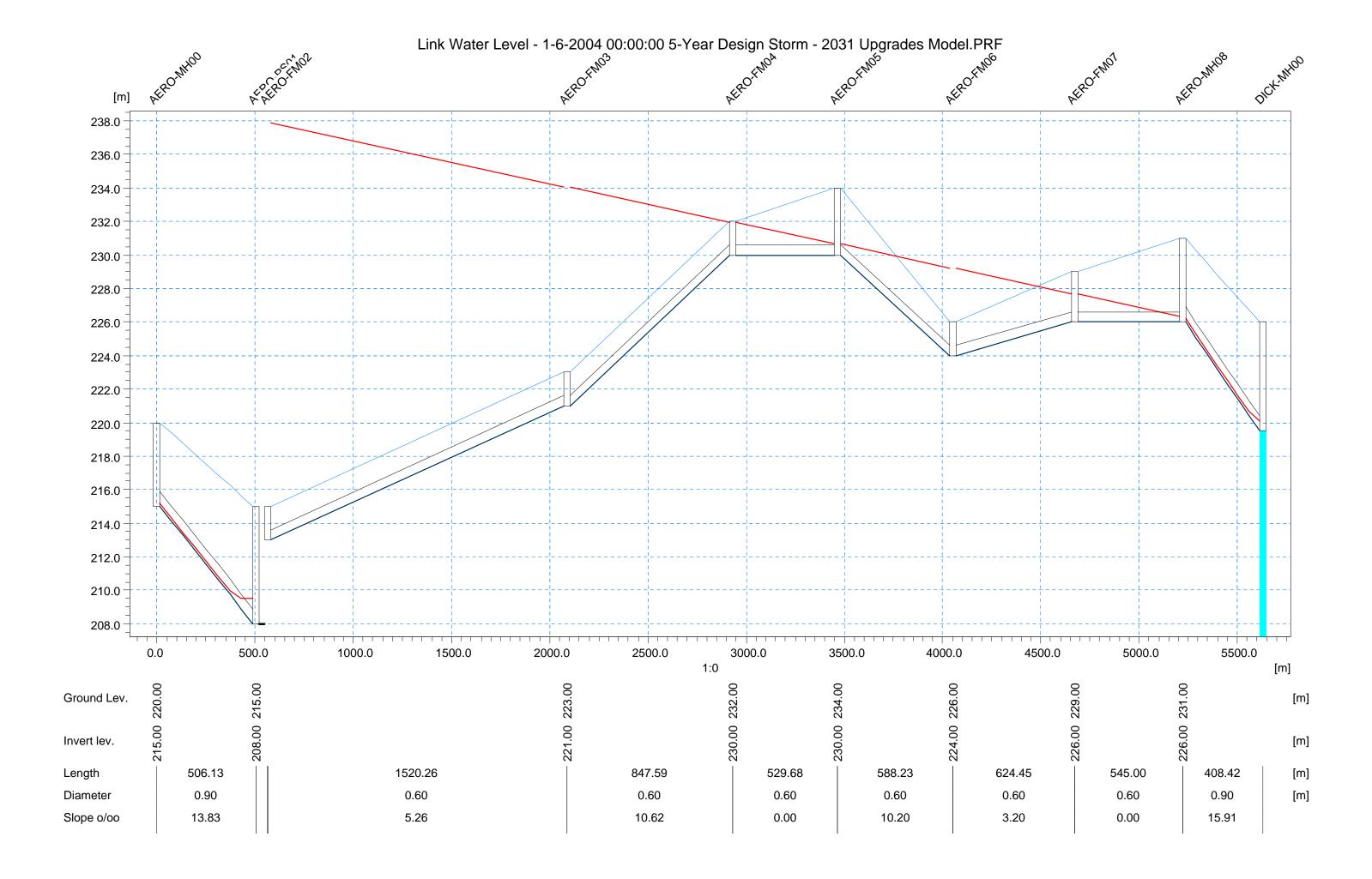
Wastewater Servicing

WW - 23 & 24 Highway 6 Forcemain



22 NOV 06 1:20,000 2590-D-82











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Appendix A-2 (18)

Projects WW-26, WW-27 and WW-28

Dickenson Road Gravity Sewer, Pumping Station and Forcemain



The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed





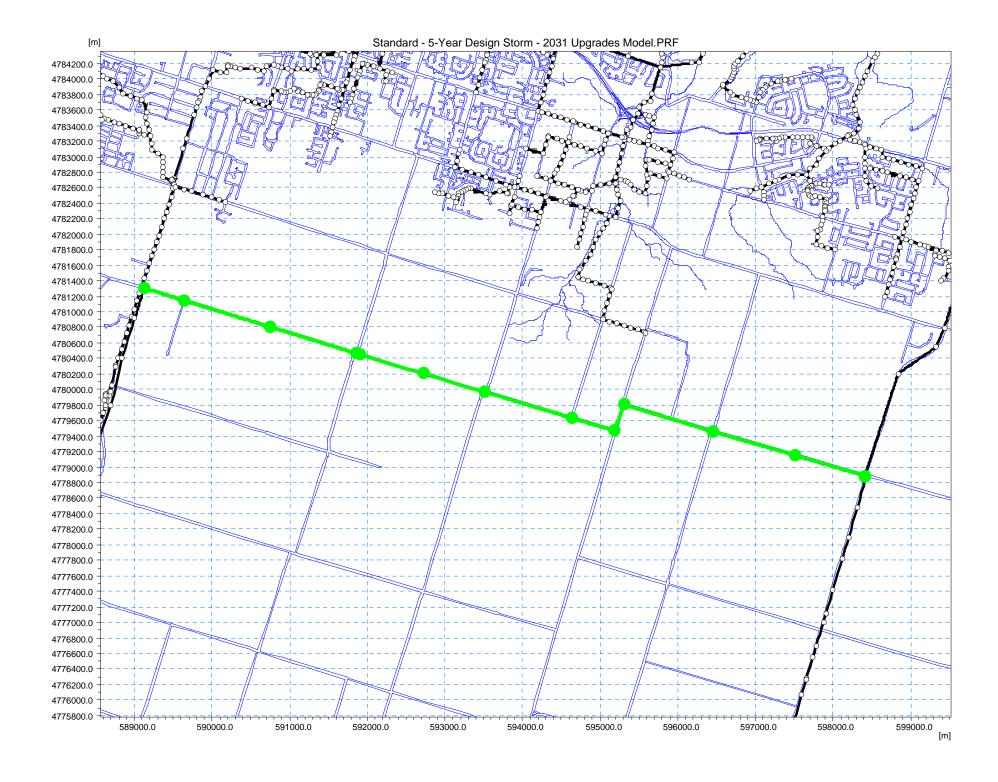
Wastewater Servicing

WW - 14,26,27 & 28 Forcemain & Gravity Sewer



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2031 Upgrades Model.PRF

Discharge	0.000	0.000	0.000	-0.000	6-2004 00:00:00 0.000	0.000	0.000	0.000	0.000	0.000	0.000	m3/s
					, noto							m3/s
f1	Dick.intoo	CKW.	OICK-MHO?	tolck finox	DiCK-FINDS (JICK-MHO6	DICK-MHO1	DICK-MHOS	Vyk.	ICK-MH10	Dick Mrh "	CMINN
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						1:0						[m]
Ground Lev.	226.00	224.00	218.00	211.00	212.00	4.00	214.00	213.00		00.500	213.00	[m]
	22	22.		0 21	21:	21	21,) 21;	Š	7	0 21:	- •
Invert lev.	219.50	218.96 8.96	214.00	204.00	210.00	210.00	208.80	208.20	Ç	00.00	204.90	[m]
Length	⊼ 537.37	⊼ │ 1157.48	1159.15	⊗ 857.74	⊼ 826.83	⊼ │ 1178.51	∺ 567.32	— 2(— 2(1194.56	ุง 1112.58	929.50	[m]
Diameter	0.90	0.90	0.90	0.60	0.60	1.20	1.20	1.20	1.20	1.20	1.20	[m]
Slope o/oo	1.00	4.29	6.04	1.17	0.00	1.02	1.06	1.12	1.51	0.99	1.00	
				П	I		l				l	



New Dickenson Road SPS Station ID: Model ID: Pumps: Firm Capacity:

2001	Catchment TZ-2645 TZ-2649 TOTAL	Status Undeveloped Undeveloped	Area [ha] 514 1170 1684.00	Pop 0 0 0	Pop Flow [L/s] 0.0 0.0 0.0	Jobs 811 2301 3112	Jobs Flow [L/s] 2.4 6.9 9.4	Avg. Flow [L/s] 2.4 6.9 9.4	Peaking Factor 4.0 4.0 4.0	Peak Flow [L/s] 9.7 27.6 37.3	Infiltration [L/s] 102.8 234.0 336.8	Total Flow [L/s] 112.5 261.6 374.1
2011	Catchment TZ-2645 TZ-2649 TOTAL	Status Undeveloped Undeveloped	Area [ha] 514 1170 1684.00	Pop 0 0 0	Pop Flow [L/s] 0.0 0.0	Jobs 938 2818 3756	Jobs Flow [L/s] 2.8 8.5 11.3	Avg. Flow [L/s] 2.8 8.5 11.3	Peaking Factor 3.8 3.8 3.8	Peak Flow [L/s] 10.8 32.5 43.4	Infiltration [L/s] 102.8 234.0 336.8	Total Flow [L/s] 113.6 266.5 380.2
2021	Catchment TZ-2645 TZ-2649 TOTAL	Status Undeveloped Undeveloped	Area [ha] 514 1170 1684.00	Pop 0 0 0	Pop Flow [L/s] 0.0 0.0	Jobs 2520 5078 7598	Jobs Flow [L/s] 7.6 15.3 22.9	Avg. Flow [L/s] 7.6 15.3 22.9	Peaking Factor 3.3 3.3 3.3	Peak Flow [L/s] 25.3 50.9 76.2	Infiltration [L/s] 102.8 234.0 336.8	Total Flow [L/s] 128.1 284.9 413.0
2031	Catchment TZ-2645 TZ-2649 TOTAL	Status Undeveloped Undeveloped	Area [ha] 514 1170 1684.00	Pop 0 0 0	Pop Flow [L/s] 0.0 0.0	Jobs 7168 10932 18100	Jobs Flow [L/s] 21.6 32.9 54.5	Avg. Flow [L/s] 21.6 32.9 54.5	Peaking Factor 2.8 2.8 2.8	Peak Flow [L/s] 60.4 92.2 152.6	Infiltration [L/s] 102.8 234.0 336.8	Total Flow [L/s] 163.2 326.2 489.4







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Appendix A-2 (19)

Projects WW-29, WW-30, WW-31 and WW-32

New Shaver Road SPS, Forcemain, and Trunk Sewer

Shaver Road Pumping Station
Station ID: HC053
Model ID: AH12PS01
Pumps: 2 @ 6 L/s each
Firm Capacity: 6 L/s

2001	Catchment AH12PS01.1 AI12A079.1 TOTAL	Status Undeveloped Undeveloped	Area [ha] 16.18 25.39 41.57	Pop 321 517 838	Pop Flow [L/s] 1.1 1.8 2.9	Jobs 0 4 5	Jobs Flow [L/s] 0.0 0.0 0.0	Avg. Flow [L/s] 1.1 1.8 2.9	Peaking Factor 5.0 5.0 5.0	Peak Flow [L/s] 5.6 9.0 14.6	Infiltration [L/s] 3.2 5.1 8.3	Total Flow [L/s] 8.8 14.1 22.9
2011	Catchment AH12PS01.1 Al12A079.1 TOTAL	Status Undeveloped Undeveloped	Area [ha] 16.18 25.39 41.57	Pop 378 608 986	Pop Flow [L/s] 1.3 2.1 3.4	Jobs 0 5 5	Jobs Flow [L/s] 0.0 0.0	Avg. Flow [L/s] 1.3 2.1 3.4	Peaking Factor 5.0 5.0 5.0	Peak Flow [L/s] 6.6 10.6 17.2	Infiltration [L/s] 3.2 5.1 8.3	Total Flow [L/s] 9.8 15.7 25.5
2021	Catchment AH12PS01.1 Al12A079.1 TOTAL	Status Undeveloped Undeveloped	Area [ha] 16.18 25.39 41.57	Pop 422 678 1100	Pop Flow [L/s] 1.5 2.4 3.8	Jobs 0 5 6	Jobs Flow [L/s] 0.0 0.0 0.0	Avg. Flow [L/s] 1.5 2.4 3.8	Peaking Factor 4.9 4.9 4.9	Peak Flow [L/s] 7.2 11.6 18.8	Infiltration [L/s] 3.2 5.1 8.3	Total Flow [L/s] 10.4 16.7 27.1
2031	Catchment AH12PS01.1 AI12A079.1 TOTAL	Status Undeveloped Undeveloped	Area [ha] 16.18 25.39 41.57	Pop 417 670 1087	Pop Flow [L/s] 1.4 2.3 3.8	Jobs 0 6 6	Jobs Flow [L/s] 0.0 0.0	Avg. Flow [L/s] 1.4 2.3 3.8	Peaking Factor 4.9 4.9 4.9	Peak Flow [L/s] 7.1 11.5 18.6	Infiltration [L/s] 3.2 5.1 8.3	Total Flow [L/s] 10.3 16.6 26.9

Shaver Road Pumping Station

Pumping Station Attributes
Station ID: HC053
Model ID: AH12PS01
Pumps: 2 @ 6 L/s each
Firm Capacity: 6 L/s

	2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Population Projections				
(A) Total Serviced Population	838	986	1100	1087
Employment Projections				
(B) Number of Jobs	5	5	6	6
Design Flow				
(C) Area	41.6 ha	41.6 ha	41.6 ha	41.6 ha
(D) Average Residential Flow (300 Lpcd)	2.9 L/s	3.4 L/s	3.8 L/s	3.8 L/s
(E) Average Employment Flow (260 Lpcd)	0.0 L/s	0.0 L/s	0.0 L/s	0.0 L/s
(F) Average Flow	2.9 L/s	3.4 L/s	3.8 L/s	3.8 L/s
(G) Peaking Factor	5.0	5.0	4.9	4.9
(H) Domestic Peak Flow, (G)*(F)	14.6 L/s	17.2 L/s	18.8 L/s	18.6 L/s
(I) External Infiltration, (C)*0.2 L/ha/s	8.3 L/s	8.3 L/s	8.3 L/s	8.3 L/s
TOTAL PEAK FLOW, (H) + (I)	22.9 L/s	25.5 L/s	27.1 L/s	26.9 L/s







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Appendix A-2 (20)

Project WW-33

Battlefield Trunk Sewer Upgrades



Note:

The proposed infrastructure has been depicted approximately for Master Planning purposes. The exact location of the proposed infrastructure will be determined in the subsequent project detailed phases.



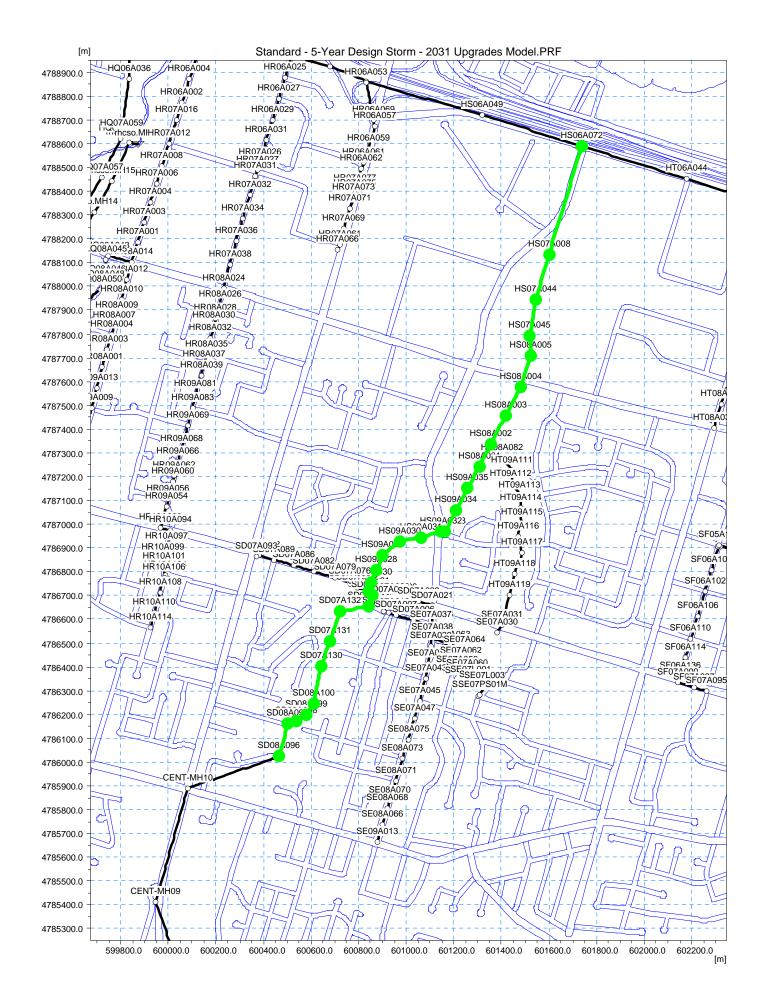


Wastewater Servicing

WW-33 Gravity Sewer



22 NOV 06 1:10,000 2590-D-86



Link Water Level - 1-6-2004 00:00:00 5-Year Design Storm - 2031 Upgrades Model.PRF 0.000/ 0.000 0.000/ 0.000 m3/s Discharge Y509030 Y509A03A y SO A COST 4508A001 Y509A031 THEODOSS SOURD HOURD HOURD [m] 92.0 90.0 88.0 86.0 84.0 82.0 80.0 78.0 76.0 74.0 72.0 70.0 68.0 66.0 200.0 400.0 600.0 800.0 2800.0 3000.0 0.0 1000.0 1200.0 1400.0 1600.0 1800.0 2000.0 2200.0 2400.0 2600.0 [m] 1:0 83.16 77.48 77.19 92.80 90.56 89.42 89.03 86.45 85.13 83.83 81.62 81.74 79.93 79.52 80.85 78.12 79.55 76.94 Ground Lev. [m] 88.30 87.38 85.82 78.96 79.55 78.57 76.08 75.53 75.01 72.08 71.37 70.99 70.31 69.97 77.61 Invert lev. [m] 474.90/ 474.90 Length [m] 1.20/ 1.20 1.20/ 1.20 | 1.50/ 1.50 1.50/ 1.50 Diameter [m] 2.10/ 2.10 Slope o/oo 2.34/ 2.34 1.69/ 1.69







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Appendix A-2 (21)

Project WW-34
Scenic Drive SPS (HC002) Upgrades

West 31st Street and Scenic Drive Pumping Station Station ID: HC002 Model ID: HD13PS01 Pumps: 2 @ 57 L/s each Firm Capacity: 57 L/s

2001	Catchment HD13PS01.1 HD13PS01.2 HD13A022.1 HF13A124.1	Status Active Active Active Active	Area [ha] 141.74 21.37 5.09 2.12	Pop 2634 580 151 29	Pop Flow [L/s] 9.1 2.0 0.5 0.1	953 44 0 2	Jobs Flow [L/s] 2.9 0.1 0.0 0.0	Avg. Flow [L/s] 12.0 2.1 0.5 0.1	9.00 Peaking Factor 3.7 3.7 3.7 3.7 3.7	Peak Flow [L/s] 44.7 8.0 1.9 0.4	Infiltration [L/s] 28.3 4.3 1.0 0.4	Total Flow [L/s] 73.0 12.3 3.0 0.8
	TOTAL	Active	170.32	3394	11.8	999	3.0	14.8	3.7 3.7	55.0	34.1	89.1
2011	Catchment	Status	Area	Рор	Pop Flow	Jobs	Jobs Flow	Avg. Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow
			[ha]		[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	HD13PS01.1	Active	141.74	2600	9.0	1016	3.1	12.1	3.7	44.9	28.3	73.3
	HD13PS01.2	Active	21.37	570	2.0	43	0.1	2.1	3.7	7.8	4.3	12.1
	HD13A022.1 HF13A124.1	Active Active	5.09 2.12	148 29	0.5 0.1	0 2	0.0 0.0	0.5 0.1	3.7 3.7	1.9 0.4	1.0 0.4	2.9 0.8
	TOTAL	Active	170.32	29 3346	11.6	∠ 1061	3.2	14.8	3.7 3.7	55.0	34.1	89.1
					Рор		Jobs		Peaking	Peak	Infiltration	Total
2021	Catchment	Status	Area [ha]	Pop	Flow [L/s]	Jobs	Flow [L/s]	Avg. Flow [L/s]	Factor	Flow [L/s]	[L/s]	Flow [L/s]
	HD13PS01.1	Active	141.74	2601	9.0	1122	3.4	12.4	3.7	45.3	28.3	73.6
	HD13PS01.2	Active	21.37	571	2.0	42	0.1	2.1	3.7	7.7	4.3	12.0
	HD13A022.1	Active	5.09	453	1.6	0	0.0	1.6	3.7	5.7	1.0	6.8
	HF13A124.1	Active	2.12	29	0.1	2	0.0	0.1	3.7	0.4	0.4	0.8
	TOTAL		170.32	3653	12.7	1166	3.5	16.2	3.7	59.1	34.1	93.2
					Pop		Jobs		Peaking	Peak	Infiltration	Total
2031	Catchment	Status	Area	Pop	Flow	Jobs	Flow	Avg. Flow	Factor	Flow		Flow
			[ha]		[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	HD13PS01.1	Active	141.74	2845	9.9	1217	3.7	13.5	3.6	49.1	28.3	77.5
	HD13PS01.2	Active	21.37	571	2.0	44	0.1	2.1	3.6	7.7	4.3	11.9
	HD13A022.1	Active	5.09	277	1.0	0	0.0	1.0	3.6	3.5	1.0	4.5
	HF13A124.1	Active	2.12	29	0.1	2	0.0	0.1	3.6	0.4	0.4	8.0
	TOTAL		170.32	3721	12.9	1263	3.8	16.7	3.6	60.6	34.1	94.7

West 31st and Scenic Drive Pumping Station

Pumping Station Attributes Station ID: HC002 Model ID: HD13PS01 Pumps: 2 @ 57 L/s each

Firm Capacity: 57 L/s				
	2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Population Projections				
(A) Total Serviced Population	3394	3346	3653	3721
Employment Projections				
(B) Number of Jobs	999	1061	1166	1263
Design Flow				
(C) Area	170.3 ha	170.3 ha	170.3 ha	170.3 ha
(D) Average Residential Flow (300 Lpcd)	11.8 L/s	11.6 L/s	12.7 L/s	12.9 L/s
(E) Average Employment Flow (260 Lpcd)	3.0 L/s	3.2 L/s	3.5 L/s	3.8 L/s
(F) Average Flow	14.8 L/s	14.8 L/s	16.2 L/s	16.7 L/s
(G) Peaking Factor	3.7	3.7	3.7	3.6
(H) Domestic Peak Flow, (G)*(F)	55.0 L/s	55.0 L/s	59.1 L/s	60.6 L/s
(I) External Infiltration, (C)*0.2 L/ha/s	34.1 L/s	34.1 L/s	34.1 L/s	34.1 L/s
TOTAL PEAK FLOW, (H) + (I)	89.1 L/s	89.1 L/s	93.2 L/s	94.7 L/s







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Appendix A-2 (22)

Project WW-35

Calvin Street SPS (HC011) Upgrades

Calvin Street Pumping Station Station ID: HC011 Model ID: AL12PS01

Model ID:	AL12P501
Pumps:	2 @ 59 L/s each
Firm Capacity:	59 L/s

2001	Catchment AL12PS01.1 AL12A006.1 TOTAL Braithwaite SPS TOTAL	Status Active Active	Area [ha] 42.61 39.19 81.8	Pop 650 551 1201	Pop Flow [L/s] 2.3 1.9 4.2	Jobs 50 39 89	Jobs Flow [L/s] 0.2 0.1 0.3	Avg. Flow [L/s] 2.4 2.0 4.4	Peaking Factor 4.8 4.8 4.8	Peak Flow [L/s] 11.4 9.7 21.1	Infiltration [L/s] 8.5 7.8 16.4	Total Flow [L/s] 20.0 17.5 37.5 37.0 74.5
2011	Catchment AL12PS01.1 AL12A006.1 TOTAL Braithwaite SPS TOTAL	Status Active Active	Area [ha] 42.61 39.19 81.8	Pop 648 566 1214	Pop Flow [L/s] 2.2 2.0 4.2	Jobs 54 44 98	Jobs Flow [L/s] 0.2 0.1 0.3	Avg. Flow [L/s] 2.4 2.1 4.5	Peaking Factor 4.7 4.7 4.8	Peak Flow [L/s] 11.4 9.9 21.4	Infiltration [L/s] 8.5 7.8 16.4	Total Flow [L/s] 19.9 17.8 37.8 37.0 74.8
2021	Catchment AL12PS01.1 AL12A006.1 TOTAL Braithwaite SPS TOTAL	Status Active Active	Area [ha] 42.61 39.19 81.8	Pop 925 758 1 682	Pop Flow [L/s] 3.2 2.6 5.8	Jobs 71 56 127	Jobs Flow [L/s] 0.2 0.2	Avg. Flow [L/s] 3.4 2.8 6.2	Peaking Factor 4.4 4.4 4.8	Peak Flow [L/s] 15.2 12.4 29.6	Infiltration [L/s] 8.5 7.8 16.4	Total Flow [L/s] 23.7 20.3 45.9 37.0
2031	Catchment AL12PS01.1 AL12A006.1 TOTAL	Status Active Active	Area [ha] 42.61 39.19 81.8	Pop 977 790 1768	Pop Flow [L/s] 3.4 2.7 6.1	Jobs 76 69 144	Jobs Flow [L/s] 0.2 0.2 0.4	Avg. Flow [L/s] 3.6 3.0 6.6	Peaking Factor 4.4 4.4 4.8	Peak Flow [L/s] 15.9 13.0 31.2	Infiltration [L/s] 8.5 7.8 16.4	Total Flow [L/s] 24.4 20.8 47.6

Calvin Street Pumping Station

Pumping Station Attributes
Station ID: HC011
Model ID: AL12PS01
Pumps: 2 @ 59 L/s each
Firm Capacity: 59 L/s

Firm Capacity: 59 L/S				
	2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Population Projections				
(A) Total Serviced Population	1201	1214	1682	1768
Employment Projections				
(B) Number of Jobs	89	98	127	144
Design Flow				
(C) Area	81.8 ha	81.8 ha	81.8 ha	81.8 ha
(D) Average Residential Flow (300 Lpcd)	4.2 L/s	4.2 L/s	5.8 L/s	6.1 L/s
(E) Average Employment Flow (260 Lpcd)	0.3 L/s	0.3 L/s	0.4 L/s	0.4 L/s
(F) Average Flow	4.4 L/s	4.5 L/s	6.2 L/s	6.6 L/s
(G) Peaking Factor	4.8	4.8	4.8	4.8
(H) Domestic Peak Flow, (G)*(F)	21.1 L/s	21.4 L/s	29.6 L/s	31.2 L/s
(I) External Infiltration, (C)*0.2 L/ha/s	16.4 L/s	16.4L/s	16.4 L/s	16.4 L/s







Report III - Master Plan Class EA Report

Appendix A-2 (23)

Projects WW-36 and WW-37

Green Road SPS (HC056) and Forcemain Upgrades

LOCATION CATCHMENTID SF03A005 SF03A005.1 SF03PS01 SF03PS01.1 SF03PS01 SF03PS01.2

Green Road

Station ID: HC056 Model ID: SF03PS01 Pumps: 2 @ 62 L/s each Firm Capacity: 62 L/s

2001	Catchment SF03A005.1	Status	Area [ha]	Pop	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow [L/s]	Peaking Factor	Peak Flow [L/s]	Infiltration [L/s]	Total Flow [L/s]
	SF03PS01.1	Active	11.39	198	0.7	72	0.2	0.9	4.1	3.7	2.3	6.0
	SF03PS01.2	Active	54.84	1562	5.4	951	2.9	8.3	4.1	33.8	11.0	44.7
	TOTAL		66.23	1760	6.1	1023	3.1	9.2	4.1	37.4	13.2	50.7
2011	Catchment	Status	Area	Pop	Pop Flow	Jobs	Jobs Flow	Avg. Flow	Peaking Factor	Peak Flow	Infiltration	Total Flow
			[ha]	•	[L/s]		[L/s]	[L/s]		[L/s]	[L/s]	[L/s]
	SF03A005.1											
	SF03PS01.1	Active	11.39	220	8.0	86	0.3	1.0	4.0	4.1	2.3	6.4
	SF03PS01.2	Active	54.84	1598	5.5	1095	3.3	8.8	4.0	35.5	11.0	46.5
	TOTAL		66.23	1818	6.3	1180	3.6	9.9	4.0	39.6	13.2	52.8
2021	Catchment SF03A005.1	Status	Area [ha]	Рор	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow [L/s]	Peaking Factor	Peak Flow [L/s]	Infiltration [L/s]	Total Flow [L/s]
	SF03PS01.1	Active	11.39	317	1.1	99	0.3	1.4	3.9	5.5	2.3	7.8
	SF03PS01.2	Active	54.84	1603	5.6	1266	3.8	9.4	3.9	37.0	11.0	47.9
	TOTAL		66.23	1920	6.7	1365	4.1	10.8	3.9	42.5	13.2	55.7
2031	Catchment SF03A005.1	Status	Area [ha]	Рор	Pop Flow [L/s]	Jobs	Jobs Flow [L/s]	Avg. Flow [L/s]	Peaking Factor	Peak Flow [L/s]	Infiltration [L/s]	Total Flow [L/s]
	SF03PS01.1	Active	11.39	317	1.1	112	0.3	1.4	3.6	5.2	2.3	7.5
	SF03PS01.2	Active	54.84	2948	10.2	1469	4.4	14.7	3.6	53.4	11.0	64.4
	TOTAL		66.23	3264	11.3	1581	4.8	16.1	3.6	58.7	13.2	71.9

Green Road

Pumping Station Attributes
Station ID: HC056
Model ID: SF03PS01
Pumps: 2 @ 62 L/s each
Firm Capacity: 62 L/s

Film Capacity. 62 L/S				
	2001 (active only)	2011	2021	2031 (Active & Undeveloped)
Population Projections				
(A) Total Serviced Population	1760	1818	1920	3264
Employment Projections				
(B) Number of Jobs	1023	1180	1365	1581
Design Flow				
(C) Area	66.2 ha	66.2 ha	66.2 ha	66.2 ha
(D) Average Residential Flow (300 Lpcd)	6.1 L/s	6.3 L/s	6.7 L/s	11.3 L/s
(E) Average Employment Flow (260 Lpcd)	3.1 L/s	3.6 L/s	4.1 L/s	4.8 L/s
(F) Average Flow	9.2 L/s	9.9 L/s	10.8 L/s	16.1 L/s
(G) Peaking Factor	4.1	4.0	3.9	3.6
(H) Domestic Peak Flow, (G)*(F)	37.4 L/s	39.6 L/s	42.5 L/s	58.7 L/s
(I) External Infiltration, (C)*0.2 L/ha/s	13.2 L/s	13.2 L/s	13.2 L/s	13.2 L/s
TOTAL PEAK FLOW, (H) + (I)	50.7 L/s	52.8 L/s	55.7 L/s	71.9 L/s







Report III - Master Plan Class EA Report

Appendix A-3

Capital Program







Report III - Master Plan Class EA Report

Appendix A-3 (01)

Water Capital Program
Master Plan Water Servicing Projects



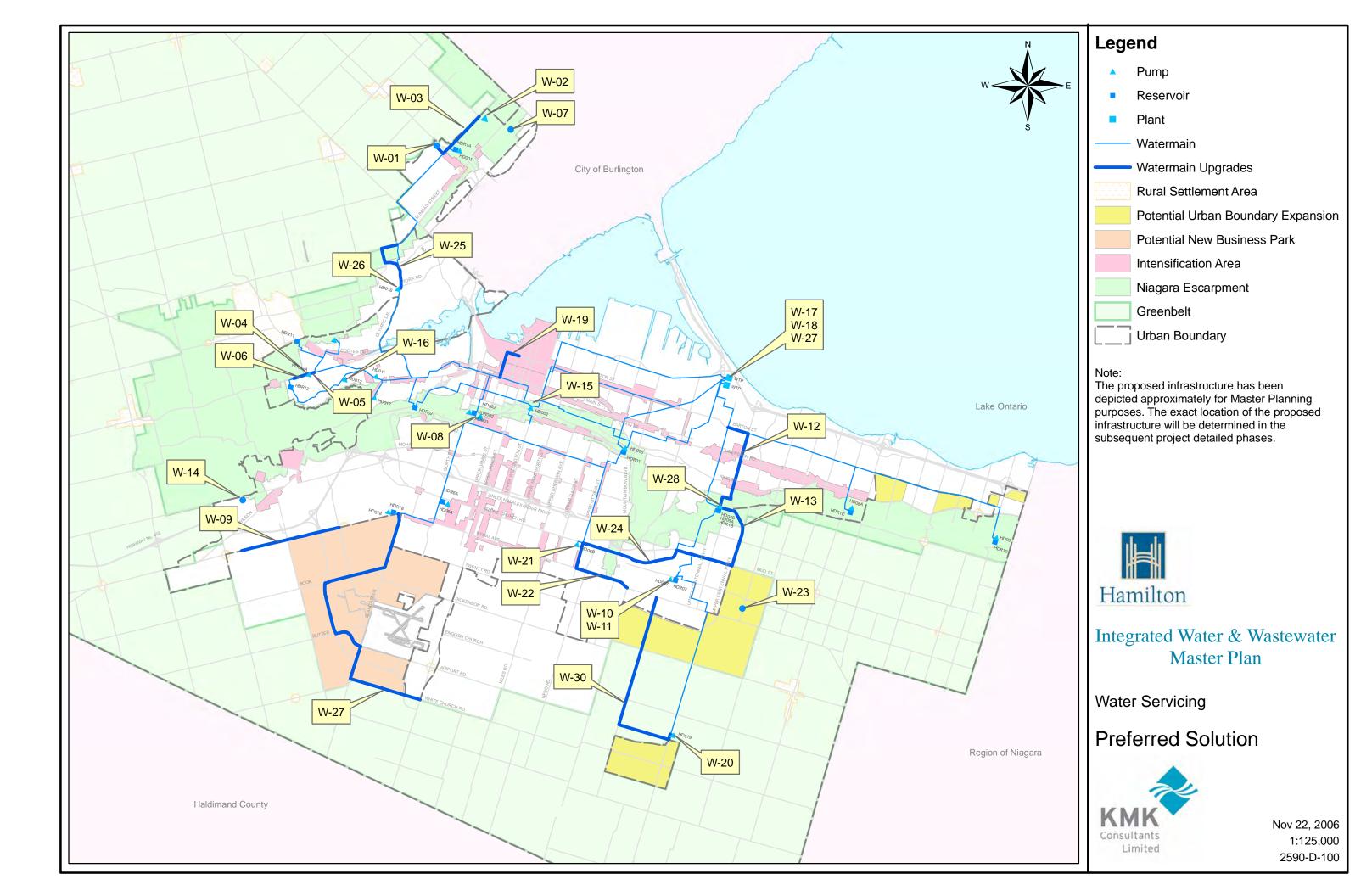
MASTER PLAN WATER SERVICING CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

Year Req'd in Service	Code	Master Plan Project No.	Type	Ward	Project	Description	Size / Total Capacity	Quantity / Additional Capacity	Total Estimated Cost (Millions)	Class EA Schedule	Trigger	Comments
2008	MP	W-01	Т	15	Waterdown North Elevated Tank	New Elevated Tank, North of Parkside Dr. and west of Centre Rd.	8 ML	8 MLD	\$6.0M	В	Growth in Waterdown North	Location is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
2008	MP	W-02	PS	15	New HD16A Pumping Station	New Pumping Station for PD16A	9.74 MLD	9.8 MLD	\$3.0M	В	Growth in UpCountry and Waterdown South	Location is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
2008	MP	W-03	F	15	Parkside Drive Watermain	New watermain on Parkside Dr. crossing Grindstone Creek and Railway Line	400 mm	2000 m	\$1.5M	В	Growth in Waterdown North	Project requires rail and river crossing. Project will support pressures and flows in Waterdown North as well as supply to new booster pumping station.
2008	MP	W-04	PS	13	HD12A Governor's Road Pumping Station Upgrades	Additional PD12 pumping capacity, new PD22 pump and new standby power	4.3 MLD	3 MLD	\$2.1M	В	Growth in Dundas, security of supply	Project required to support peak demand needs for PD12 and PD22
2008	MP	W-05	F	13	Governor's Road PD11 Watermain Extension	Twin watermain feeding HD12A	400 mm	220 m	\$0.2M	A	Growth in Dundas, security of supply	Based on the potential peak flow from HD012A, additional supply-side distribution capacity is required
2008	MP	W-06	F	13	Governor's Road PD22 Watermain Extension	New Watermain from HD12A to PD22 on Governor's Road and Moss Blvd.	300 mm	1000 m	\$0.7M	А	Growth in Dundas, security of supply	This feed will provide additional supply security to PD22
2009	MP	W-07	T	15	Waterdown South Elevated Tank	New Elevated Tank, South of Dundas St E and west of Kerns Rd.	6.3 ML	6.3 ML	\$4.5M	В	Growth in UpCountry and Waterdown South	Location is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
2009	MP	W-08	PS	1	New HD03B Highland Gardens Pumping Station	New Pumping Station for PD3	7 MLD	7 MLD	\$4.0M	В	Peak demands and security of supply in PD3	Project is being completed under independent Class EA Study
2009	MP	W-09	F	12	Garner Road Watermain	New watermain from Southcote on Garner Rd East to Wilson Rd.	500 mm	5100 m	\$6.2M	A	Peak demands and system pressures in PD18	Project required to support peak demand needs for PD18 and maintain system pressures
2009	MP	W-10	PS	9	HD007 Highland Pumping Station Upgrades	Pumping Station expansion including additional pumping capacity and new standby power	32 MLD	10 MLD	\$6.9M	В	Growth in ROPA9 and Southeast Mountain	Project was confirmed through independent Class EA Study. PS Upgrades will also address new PD7 HGL and PRVs.
2009	MP	W-11	Т	9	HD007 Highland Reservoir Expansion	HD007 Reservoir expansion	27 ML	16 ML	\$8.2M	В	Growth in ROPA9 and Southeast Mountain	Project was confirmed through independent Class EA Study
2010	MP	W-12	F	5	Centennial Pkwy Trunk Feedermain	New watermain on Centennial Parkway from Barton Street to HD05A	1200 mm	3500 m	\$11.3M	В	Growth in Hamilton Mountain, Ancaster, Airport Lands, Binbrook, security of supply	This feed will provide additional capacity and supply security to PD5 which ultimately feeds Southeast Mountain, Airport Lands and Binbrook
2010	MP	W-13	F	5,9	Centennial Pkwy Trunk Feedermain	New watermain on Centennial Parkway from HD05A up escarpment to Mud St. W	1200 mm	3000 m	\$9.9M	В	Growth in Hamilton Mountain, Ancaster, Airport Lands, Binbrook, security of supply	This feed will provide additional capacity and supply security to PD5 which ultimately feeds Southeast Mountain, Airport Lands and Binbrook
2011	MP	W-14	Т	12	Pressure District 18 Elevated Tank	New Elevated Tank, on Jerseyville Road	7 ML	7 ML	\$5.3M	В	Growth in Ancaster, security of supply	Project required to equalize system pressures and provide additional system storage
2011	MP	W-15	PS	2	HD002 Ferguson Pumping Station Upgrades (Standby Power)	New Standby Power	1000 kW	1000 KW	\$1.5M	В	Security of supply to PD2, 3	As a critical pumping station for supply to PD2, 3, standby power is required for security of supply
2011	MP	W-16	PS	13	HD012 Lynden Ave Pumping Station Upgrades	Additional pumping capacity and new standby power	4.3 MLD	3 MLD	\$2.1M	В	Growth in Dundas, security of supply	This pumping station will continue to provide a secondary feed to PD12. Based on the isolated service area, this pumping station should be maintained in service and upgraded
2011	MP	W-17	WTP	4	Sedimentation Tank upgrades	Woodward Ave. WTP Improvements			\$15.0M	A	Growth in Hamilton	Process upgrades are required to maintain rated capacity of the WTP
2011	MP	W-18	WTP	4	Pre-Chlorination system upgrades	Woodward Ave. WTP Improvements			\$1.0M	A	Growth in Hamilton	Process upgrades are required to maintain rated capacity of the WTP
2014	MP	W-19	F	9	Locke St Watermain	New watermain on Locke St N from Barton St to Main St	400 mm	1500 m	\$1.4M	A	Intensification in downtown Hamilton	Required to provide additional distribution capacity
2014	MP	W-20	PS	11	HD019 Binbrook/Hwy56 Pumping Station Upgrades	Additional pumping capacity	15 MLD	9 MLD	\$6.4M	В	Growth in Binbrook	Required to provide additional pumping capacity to PD19



MASTER PLAN WATER SERVICING CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

Year Req'd in Service		Master Plan Project No.	Туре	Ward	Project	Description	Size / Total Capacity	Quantity / Additional Capacity	Total Estimated Cost (Millions)	Class EA Schedule	Trigger	Comments
2016	MP	W-21	PS		HD06B Tunbridge Pumping Station Upgrades (HD07A)	New pumping capacity and suction/discharge piping upgrades for PD7	50 MLD	50 MLD	\$3.5M	В	Growth in ROPA9 and Southeast Mountain	This project will provide additional pumping capacity and supply security to PD7
2016	MP	W-22	F	6,9,11	HD07A Feedermain	New watermain from HD06B on Anchor Rd. to Rymal Rd. then on Rymal Rd. to Trinity Church Rd.	750 mm	3100 m	\$5.7M	A	Growth in ROPA9 and Southeast Mountain	This project will provide additional supply distribution capacity and supply security to PD7
2016	MP	W-23	T	11	Pressure District 7 Elevated Tank	New Elevated Tank, south of Highland Rd and west of First Rd. E	7 ML	7 ML	\$5.3M	В	Growth in ROPA9 and Southeast Mountain	This project will provide additional storage capacity and supply security to PD7. Location will be reviewed as part of Secondary Plan.
2016	MP	W-24	F	6,9	Stone Church Trunk Feedermain	New watermain from Centennial on Mud St W, Paramount Drive, Stone Church Rd to HD06B	1200 mm	7500 m	\$22.1M	В	Growth in ROPA9 and Southeast Mountain	Based on the additional flows from HD06B, additional supply-side distribution capacity is required
2019	MP	W-25	F	13,15	HD016 Trunk Feedermain	New watermain up Valley Rd. and Rock Chapel Rd. to Dundas St/Algonquin Ave.	600 mm	3900 m	\$6.7M	В	Growth in Waterdown and security of supply	This project is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process. It is required for security of supply in order to offset additional storage.
2019	MP	W-26	PS		HD016 York/Valley Road Pumping Station upgrades	Additional pumping capacity and new standby power	20.52 MLD	3 MLD	\$6.0M	В	Growth in Waterdown and security of supply	This project is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process. It is required for security of supply in order to offset additional storage.
2021	MP	W-27	F	11,12	Airport Lands Trunk Watermain	New Airport lands Trunk Watermain	400 mm	12200 m	\$11.7M	Α	Growth in the Airport Lands	The alignment will need to be updated as the Secondary Plan for this area is completed. An internal trunk watermain will be required to support servicing for this area.
2022	MP	W-28	PS	5	HD05A Greenhill Pumping Station Upgrades	Additional pumping capacity and new standby power	155 MLD	57 MLD	\$5.0M	В	Growth in Southeast Mountain, Ancaster, Airport Lands, Binbrook, security of supply	Additional supply capacity and supply security is required for PD5 which ultimately feeds Southeast Mountain, Airport Lands and Binbrook
2023	MP	W-29	PS/WTP	4	Woodward Ave. WTP Expansion	Additional high lift pumping capacity	592 MLD	23 MLD	\$2.0M	С	Growth in Hamilton	Additional high lift pumping capacity is required to meet buildout growth needs
2023	MP	W-30	F	9	Binbrook Trunk Feedermain	New watermain from PD7 to HD019 along Fletchers Road and Cemetary Road	400 mm	7200 m	\$5.5M	A	Growth in Binbrook, security of supply	A secondary feed is required to provide additional distribution capacity and security of supply to Binbrook. This feed could be coordinate with a future booster pumping station.
					Total Cost				\$171.0M			









Report III - Master Plan Class EA Report

Appendix A-3 (02)

Water Capital Program

Development Related Water System Improvements



DEVELOPMENT-RELATED WATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

The following capital program was identified under the City of Hamilton Development Charge Background Study completed in 2004 by C. N. Watson in association with Earth Tech Canada and Philips Engineering. The capital program has been reviewed in coordination with the development of the Master Plan capital program. The total estimated costs have bee updated to reflect 2006 dollars.

Year Reg'd in	Code	DC	Project	Description	Size / Capacity	Quantity	Total Estimated	Comments
Service		Project No.				,	Cost (Millions)	
		110.						
0-5 Years	DC	W1	North Waterdown Tower	New 12 ML Elevated Water Tower				Covered by Master Plan Project No. W-01
0-5 Years	DC	W2	North Waterdown	New 400 mm WM through North Waterdown	400 mm	2300 m	\$2.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	W3	Up Country Estates	New 400 mm WM from Parkside Dr. to Hwy. 5	400 mm	925 m	\$0.9M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	W4	Parkside Dr	New 400 mm WM from Main St. to Upcountry Estates.				Covered by Master Plan Project No. W-03
0-5 Years	DC	W5	Water Tower	New Pressure District Water Tower @ Hwy. 5 and Upcountry Estates				Covered by Master Plan Project No. W-07
0-5 Years	DC	W6	South Waterdown	New 400 mm WM from Upcountry to New Pressure District Water Tower	400 mm	475 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	W7	South Waterdown	New 400 mm WM from 470 m west of Kerns Rd. to 1070m west of Kerns Rd.	400 mm	600 m	\$0.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	W8	South Waterdown	New 400 mm WM from Flanders Dr. to 1300m east of Flanders Dr.	400 mm	1300 m	\$1.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	W9	Watermain	New 300 mm WM from North Waterdown Tower to 500m East of Hwy. 6	300 mm	1000 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	W10	Booster Station Upgrade	Upgrade of HD016 Booster Pumping Station from 208 L/s to 309 L/s				Covered by Master Plan Project No. W-26
6 Years to UBBO	DC	W11	W-H2 PS Replacement	P.S. W-H2 (replace existing)				The Master Plan has reviewed all required pumping capacity upgrades
0-5 Years	DC	A1	Garner Rd. Watermain	New 500 mm WM from Fiddlers Green Rd. to Hamilton Dr.				Covered by Master Plan Project No. W-09
0-5 Years	DC	A2	Garner Rd. Watermain	New 600 mm WM from Southcote Rd. to Fiddlers Green Rd.				Covered by Master Plan Project No. W-09
0-5 Years	DC	A3	Garner Rd. Watermain	New 500 mm WM from Hamilton Dr. to Shaver Rd.				Covered by Master Plan Project No. W-09
0-5 Years	DC	A4	Meadowlands - Phase 7B	New 400 mm WM From Meadowlands Phase 7A to Garner Rd	400 mm	500 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A5	Meadowlands- Neighbourhood 4	New 400 mm WM from Stonehenge St. to Garner Rd	400 mm	1200 m	\$1.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A6	Ancaster Industrial Park - Tradewind Dr.	New 300 mm WM from Sandhill Dr. to Cormorant	400 mm	370 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A7	Ancaster Ind. Park- Cormorant Dr. Extension (west)	New 300 mm WM from Tradewind Dr. to 400 m west of Tradewind Dr.	300 mm	400 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A8	Ancaster Ind. Park- Cormorant Dr. Extension (west)	New 300 mm WM from 400 m west of Tradewind Dr. to Trinity Rd. South	300 mm	400 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A9	Ancaster Ind. Park- Cormorant Dr. Extension (east)	New 300 mm WM from Bittern to 370 m east of Bittern	300 mm	370 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A10	Ancaster Ind. Park Trinity Rd South	New 300 mm WM from Claybar Rd. to Wilson St.	300 mm	700 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	A11	Kitty Murray Lane	New 400 mm WM from Holkem to Garner Rd.	400 mm	750 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Ancaster Industrial Park	New 300 mm WM from Shaver Rd. to 600 m west of Shaver Rd.	300 mm	600 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO			Ancaster Industrial Park	New 400 mm WM from Shaver Rd. to 600 m west of Shaver Rd.	400 mm	200 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
	DC							
6 Years to UBBO	DC		Ancaster Industrial Park	New 400 mm WM from Garner Rd. to Cormorant Dr. Extension	400 mm	700 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Pumping Station W-H18	Additional Pump				The Master Plan has determined that additional HD018 pumping capacity is not required
6 Years to UBBO	DC		Reservoir W-H18	Additional Storage				Additional PD18 storage is covered by Master Plan Project No. W-14
5 Years to UBBO	DC		Shaver Road	New 300 mm WM on Shaver Rd.	300 mm	700 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC		SW Quadrant of Binbrook Rd./Hwy 56	New 400 mm WM from 550 m west of Hwy. 56 to 700 m south of Binbrook Rd.	400 mm	1530 m	\$1.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	B2	Binbrook Rd.	New 400 mm WM from existing elevated tank to Fletcher Rd.	400 mm	1700 m	\$1.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	В3	Binbrook Looping - Fletcher Rd	New 400 mm Watermain from Binbrook Rd. to Golf Club Rd. /Trinity Church Rd.				Covered by Master Plan Project No. W-30
6 Years to UBBO	DC	B4	Reservoir P.S. & Feedermain	On Fletcher Rd. 580 m north of Binbrook Rd.			\$4.0M	Additional pumping capacity to Binbrook has been covered by Master Plan Project No. W-20. Additional booster pumping capacity should be reviewed as development proceeds.



DEVELOPMENT-RELATED WATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

The following capital program was identified under the City of Hamilton Development Charge Background Study completed in 2004 by C. N. Watson in association with Earth Tech Canada and Philips Engineering. The capital program has been reviewed in coordination with the development of the Master Plan capital program. The total estimated costs have bee updated to reflect 2006 dollars.

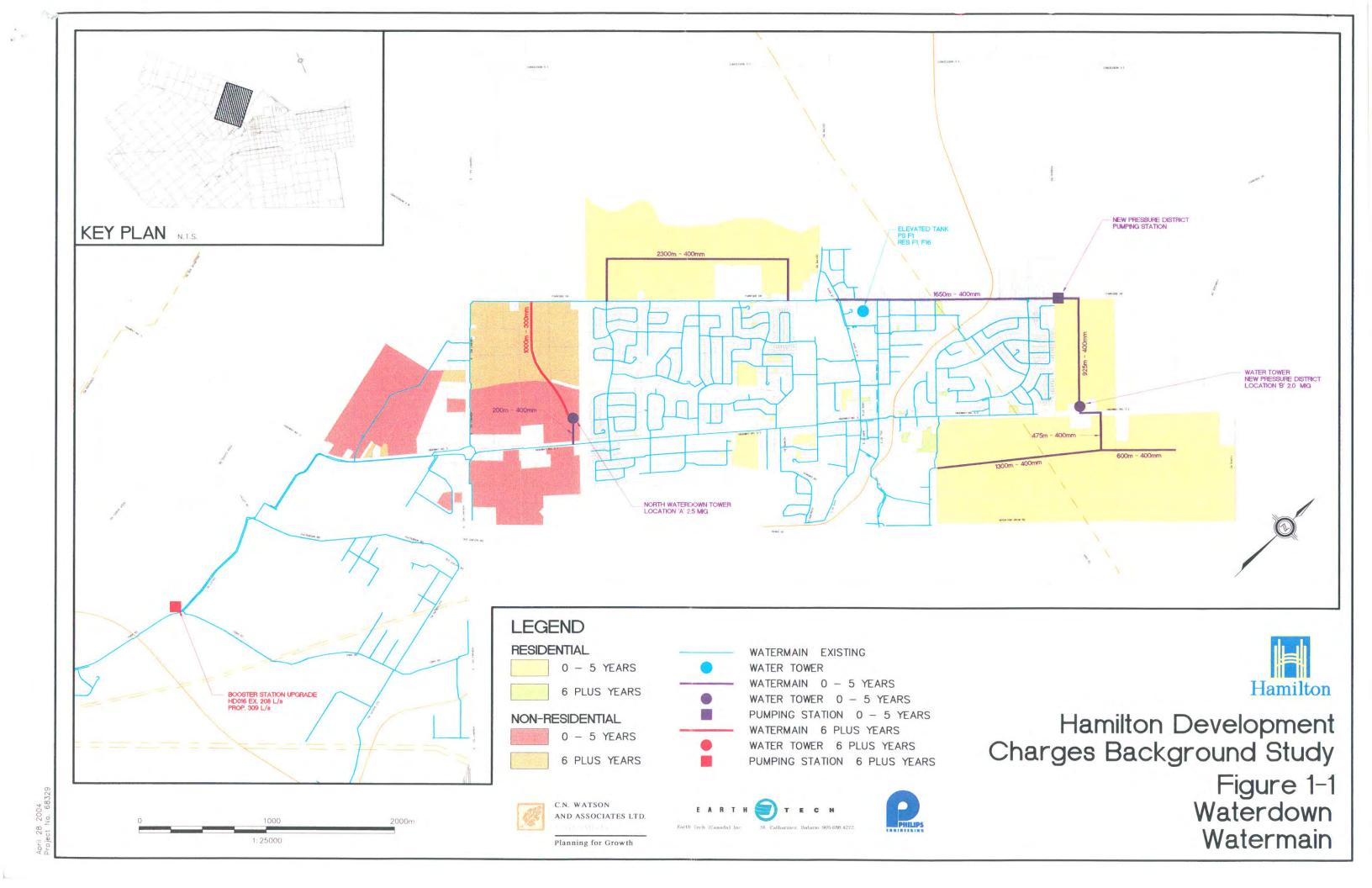
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Year Req'd in Service	Code	Project	Project	Description	Size / Capacity	Quantity	Total Estimated Cost (Millions)	Comments
		No.						
0-5 Years	DC	MH1	White Church Rd. E	New 150 mm WM from 250 m west of Hwy. 6 to 590 m west of Hwy. 6				Covered by Master Plan Project No. W-27
0-5 Years	DC	MH2	Airport Looping Trunk WM - Airport Rd.	New 500 mm WM from 1700 m east of Glancaster to Glancaster				Covered by Master Plan Project No. W-27
0-5 Years	DC	MH3	Airport Looping Trunk WM - Glancaster Rd.	New 600 mm WM from Airport Rd. to Twenty Rd.				Covered by Master Plan Project No. W-27
0-5 Years	DC	MH4	Airport Looping Trunk WM - Twenty Rd.	New 600 mm WM from 1165 m East of Glancaster Rd. to Glancaster Rd.				Covered by Master Plan Project No. W-27
0-5 Years	DC	MH5	Airport Industrial Park	New 300 mm WM from 700 m west of Homestead Dr. to English Church Rd.	300 mm	2000 m	\$1.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	MH6	Airport Water Reservoir					Pumping and storage capacity for PD6/18 has been confirmed under the Master Plan analysis
6 Years to UBBO	DC	MH7	Deferral 11				\$4.0M	Servicing needs and timing to be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	MH8	SPA 1	From Dickenson to North Airport Property Line			\$2.0M	Servicing needs and timing to be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	MH9	SPA 2	From Airport Rd to Hwy 6			\$2.5M	Servicing needs and timing to be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM1	Crerar Dr.	New 400 mm WM from 160 m North of Stone Church Rd. to Stone Church Rd.	400 mm	160 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM2	Acadia Dr	From Upper Sherman to n/s leg Acadia Dr.	400 mm	320 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM3	Terni Blvd./Cadham Blvd.	New 400 mm WM from 125 m west of Upper Gage to Miles Rd.	400 mm	370 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM4	Extension of Terni Blvd.	New 400 mm WM from Miles Rd. to 300 m west of Miles Rd.	400 mm	300 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM5	Upper Wentworth St./ Twenty Rd.	New 400 mm WM from South End of Upper Wentworth St. to Highway 6	400 mm	2450 m	\$2.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM6	Vineberg Dr.	New 400mm WM from 140m east of Upper Wentworth to 590 m east of Upper Wentworth	400 mm	450 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM7	Shermal Estates	New 600mm WM from Stone Church Rd. to Rymal	600 mm	1000 m	\$1.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM8	Portland Dr.	New 300mm WM from Pritchard Rd. to Anchor Rd.	300 mm	220 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0-5 Years	DC	HM9	Ditton Dr. Extension	New 200mm WM from Hempstead to Lancing	200 mm	680 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	HM10	Terni Blvd. Extension	New 400 mm WM from 590 m east of Upper Wentworth to 300 m west of Miles Rd.	400 mm	450 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	HM11	P.S. W-H6B Additional Pump					Pumping and storage capacity for PD6 has been confirmed under the Master Plan analysis
6 Years to UBBO	DC	HM12	Reservoir W-H5 Additional Storage					Pumping and storage capacity at the Stonechurch pumping station and reservoir has been confirmed under the Master Plan analysis and no
6 Years to UBBO	DC	HM13	First Rd. W, Rymal, Upper Ottawa, Tunbridge	New 1200 mm WM from P.S. W-H6B to P.S. W-H5A				reservoir expansion is required Covered by Master Plan Project No. W-13 and W-24
6 Years to UBBO	DC		Upper Ottawa, Hydro Corr., Christie St. Fifth St. Stone Church Rd.	New 1200 mm WM from Rymal Rd. East to P.S. W-H6A	1200 mm	7800 m	\$23.0M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	HM15	P.S. W-H5A	Add Pump #5	877 L/s		\$0.7M	Covered by Master Plan Project No. W-28
6 Years to UBBO	DC	HM16	P.S. W-H1 Additional Pumps		2625 L/s		\$2.2M	Covered by Master Plan Project No. W-23
6 Years to UBBO	DC	HM17	P.S. W-H5A	Add Pump #3	877 L/s		\$0.7M	Covered by Master Plan Project No. W-28
6 Years to UBBO	DC		Reservoir H1B					Pumping and storage capacity at the Greenhill pumping station and reservoir has been confirmed under the Master Plan analysis and no
6 Years to UBBO				New 400 mm WM from Glover Rd. to Trinity Church Rd.	400 mm	580 m	\$0.6M	reservoir expansion is required
	DC		Twenty Rd.	New 400 mm WM from Glover Rd. to Trinity Church Rd.	400 mm	580 m	\$0.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Twenty Rd.	New 400 mm WM from Glover Rd. to 680 m west of Glover Rd.	400 mm	680 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Twenty Rd.	New 400 mm WM from 680 m west of Glover Rd. to Nebo Rd.	400 mm	500 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Twenty Rd.	New 300mm WM from Nebo Rd. to 900m west of Nebo Rd.	300 mm	900 m	\$0.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC		Nebo Rd	New 400 mm WM from Twenty Rd. to Dickinson Rd.	400 mm	1350 m	\$1.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	HM24	Dickinson Rd.	New 200mm WM from Nebo Rd. to 800 mm east of Nebo Rd.	200 mm	800 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements

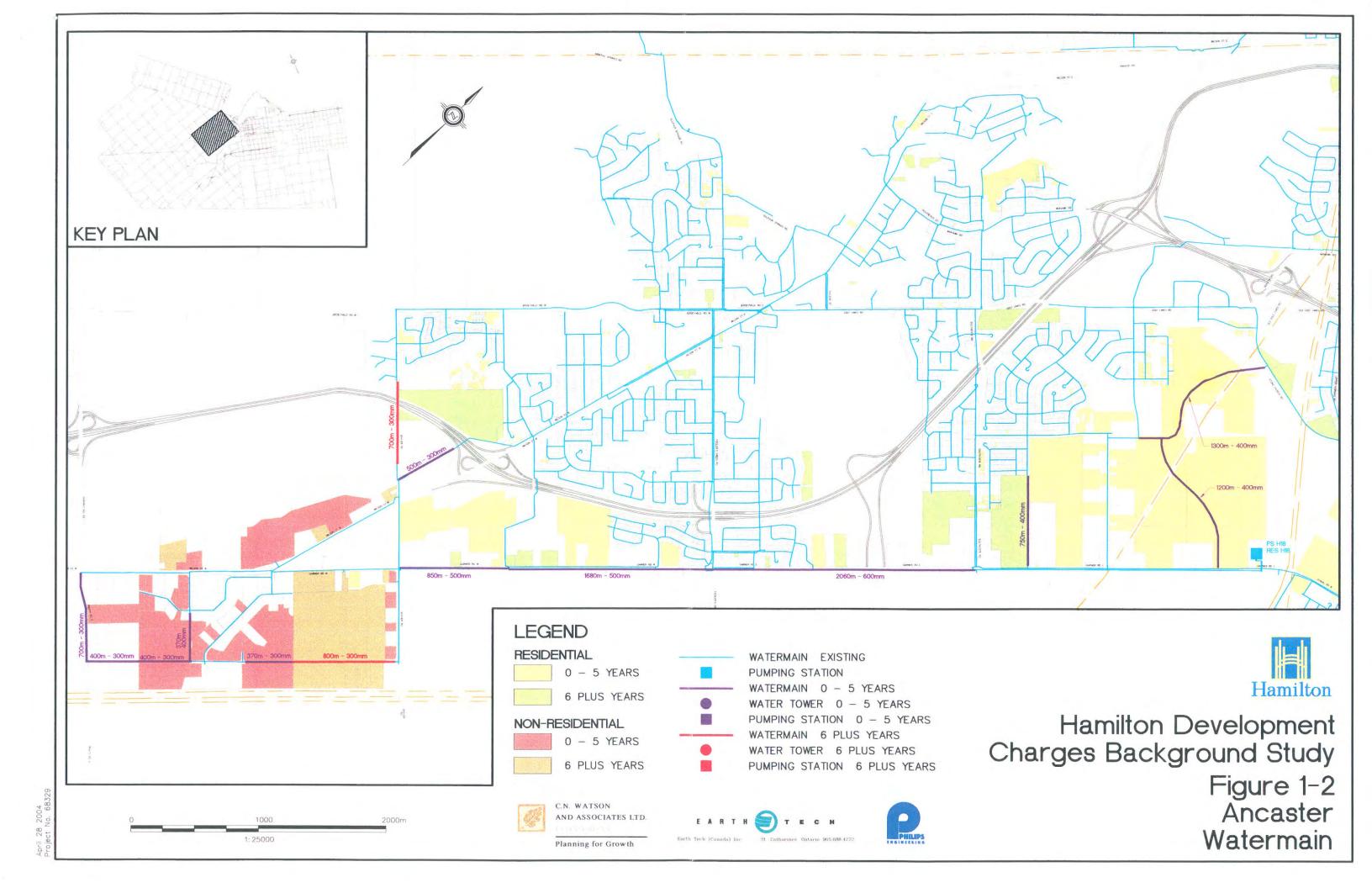


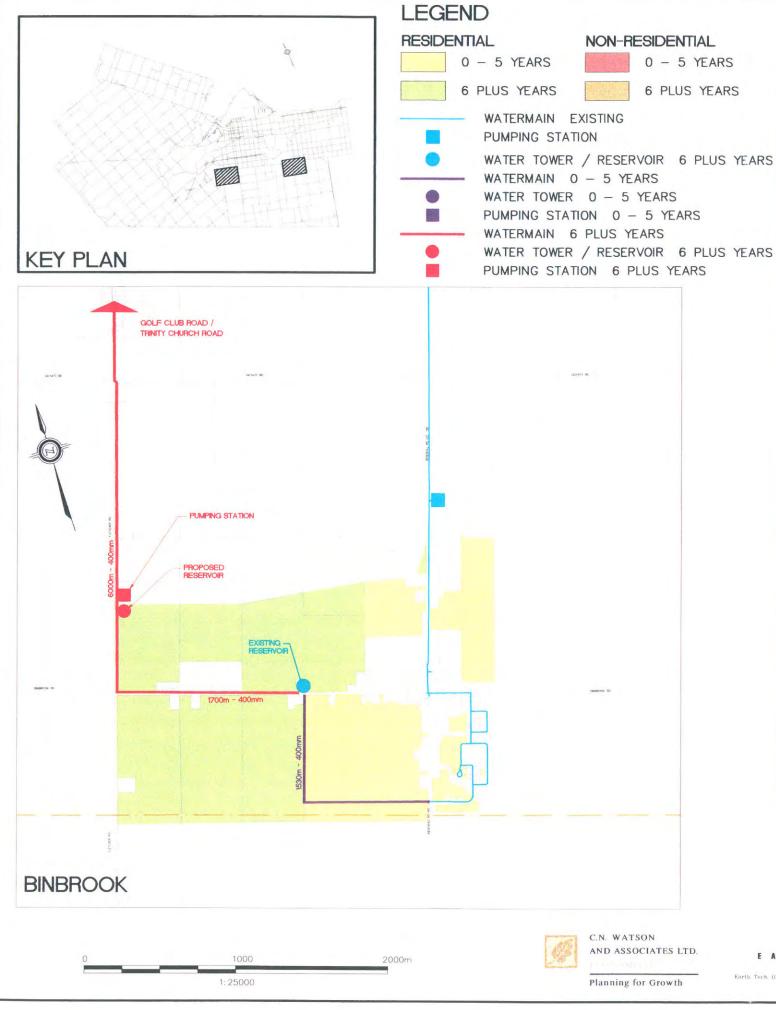
DEVELOPMENT-RELATED WATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

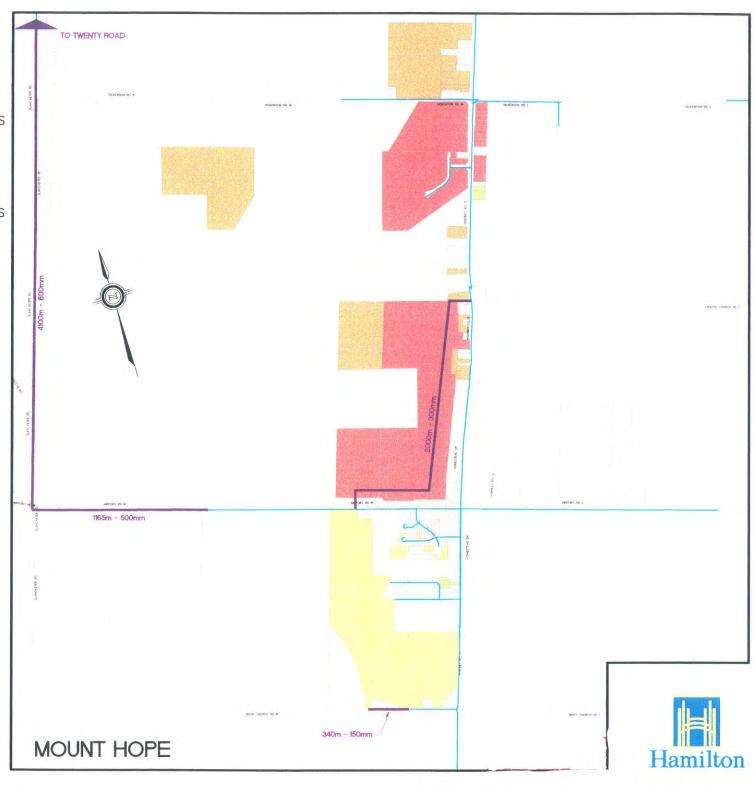
The following capital program was identified under the City of Hamilton Development Charge Background Study completed in 2004 by C. N. Watson in association with Earth Tech Canada and Philips Engineering. The capital program has been reviewed in coordination with the development of the Master Plan capital program. The total estimated costs have bee updated to reflect 2006 dollars.

Year Req'd in	Code	DC	Project	Description	Size / Capacity	Quantity	Total Estimated	Comments
Service		Project No.					Cost (Millions)	
-5 Years	DC	SCU1	ROPA-9 Area-Summit Park East	New 400mm WM from Fletcher Rd. to 815 m east of Fletcher Rd.	400 mm	800 m	\$0.8M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
-5 Years	DC	SCU2	ROPA-9 Area-Summit Park West	New 400 mm WM from Fletcher Rd. to Trinity Church Rd.	400 mm	1200 m	\$1.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
-5 Years	DC	SCU3	North Extension of Isaac Brock Dr.	New 300 mm WM from Green Mountain Rd. to Escarpment	300 mm	375 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCU4	Green Mountain Rd.	New 400 mm WM from Isaac Brock Dr. to First Rd. W	400 mm	750 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years	DC	SCU5	First Rd. West	New 300 mm WM from Green Mountain Rd. to Escarpment	300 mm	850 m	\$0.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCU6	Nash Planning Neighbourhood	New 400 mm WM from North Extension of Isaac Brock Dr. to First Rd. W	400 mm	850 m	\$0.8M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCU7	Reservoir W-H7 Additional Storage					Covered by Master Plan Project No. W-11. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
-5 Years	DC	SCU8	P.S. W-H7 Pump Replacement					Covered by Master Plan Project No. W-10. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
-5 Years	DC	SCU9	Upper Mount Albino Rd. Extension	New 300 mm WM from 235 m south of Highland Rd. W to Highland Rd W	300 mm	235 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCU10	Booster Station in ROPA - 9					Covered by Master Plan Project No. W-10. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
-5 Years	DC	SCU11	Heritage Green	New 600 mm WM from Reservoir H7 (Highland Rd.) to 150 m south of H7	600 mm	150 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCU12	ROPA - 9 Area- The crossing/Brooks at Rymal	New 400 mm WM from 800m east of Fletcher Rd. to Swayze Rd.	400 mm	1250 m	\$1.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. Servicing requirements have also been reviewed as part of the ROPA9 Class EA.
Years to UBBO	DC	SCU13	Heritage Green	New 400 mm WM from 200 m south of Highland Rd. to 300 m north of Rymal Rd.	400 mm	945 m	\$0.9M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. Environmental concerns including construction through Karst will need to be confirmed.
Years to UBBO	DC	SCU14	Heritage Green	New 400 mm WM from 200 m north of Rymal Rd. to 2nd Rd. west, 200 m north of Rymal Rd.	400 mm	900 m	\$0.9M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. Environmental concerns including construction through Karst will need to be confirmed.
-5 Years	DC	SCL1	Winona Rd	New 400 mm WM from Hwy 8 to Barton	400 mm	440 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL2	Glover Rd.	New 300 mm WM from Lakeshore Rd. to South Service Rd.	300 mm	420 m	\$0.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL3	Fifty Rd.	New 400 mm WM from Barton St. To North Service Rd.	400 mm	1400 m	\$1.3M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL4	Arvin Ave. Extension	New 300 mm WM from 350 m west of McNeilly Rd. to McNeilly Rd	300 mm	350 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL5	Arvin Ave. Extension	New 300 mm WM from 550 m east of Jones Rd. to Jones Rd.	300 mm	550 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL6	Arvin Ave. Extension	New 300 mm WM McNeilly Rd. to 335 m west of Winona Rd	300 mm	1500 m	\$1.1M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL7	Replacement on Hwy. 8	New 400 mm WM from 420 m east of Glover Rd. to NcNeilly Rd.	400 mm	450 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL8	Replacement on Hwy. 8	New 300 mm WM from McNeilly to Lewis Rd.	300 mm	1000 m	\$0.7M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL9	Replacement on Lewis Rd.	New 300 mm WM from Hwy. 8 to Barton St.	300 mm	500 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL10	McNeilly Rd.	New 300 mm WM from Barton St to Railway	300 mm	550 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
-5 Years	DC	SCL11	Glover Rd.	New 300 mm WM from Barton St. to Service Rd. Extension	300 mm	700 m	\$0.5M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCL12	Millen Rd.	New 400 mm WM from South Service Rd. to Barton St.	400 mm	1000 m	\$1.0M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCL13	South Service Rd.	New 400 mm WM from Jones Rd. to Millen Rd.	400 mm	2745 m	\$2.6M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCL14	Dewitt Rd.	New 300 mm WM from CNR Tracks to Barton St.	300 mm	610 m	\$0.4M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCL15	Jones Rd.	New 400 mm WM from South Service Rd. to Barton St.	400 mm	915 m	\$0.9M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	SCL16	Winona Rd	New 300 mm WM from Service Rd. to Petit Rd.	300 mm	250 m	\$0.2M	The watermain alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
			Total Cost				\$84.8M	







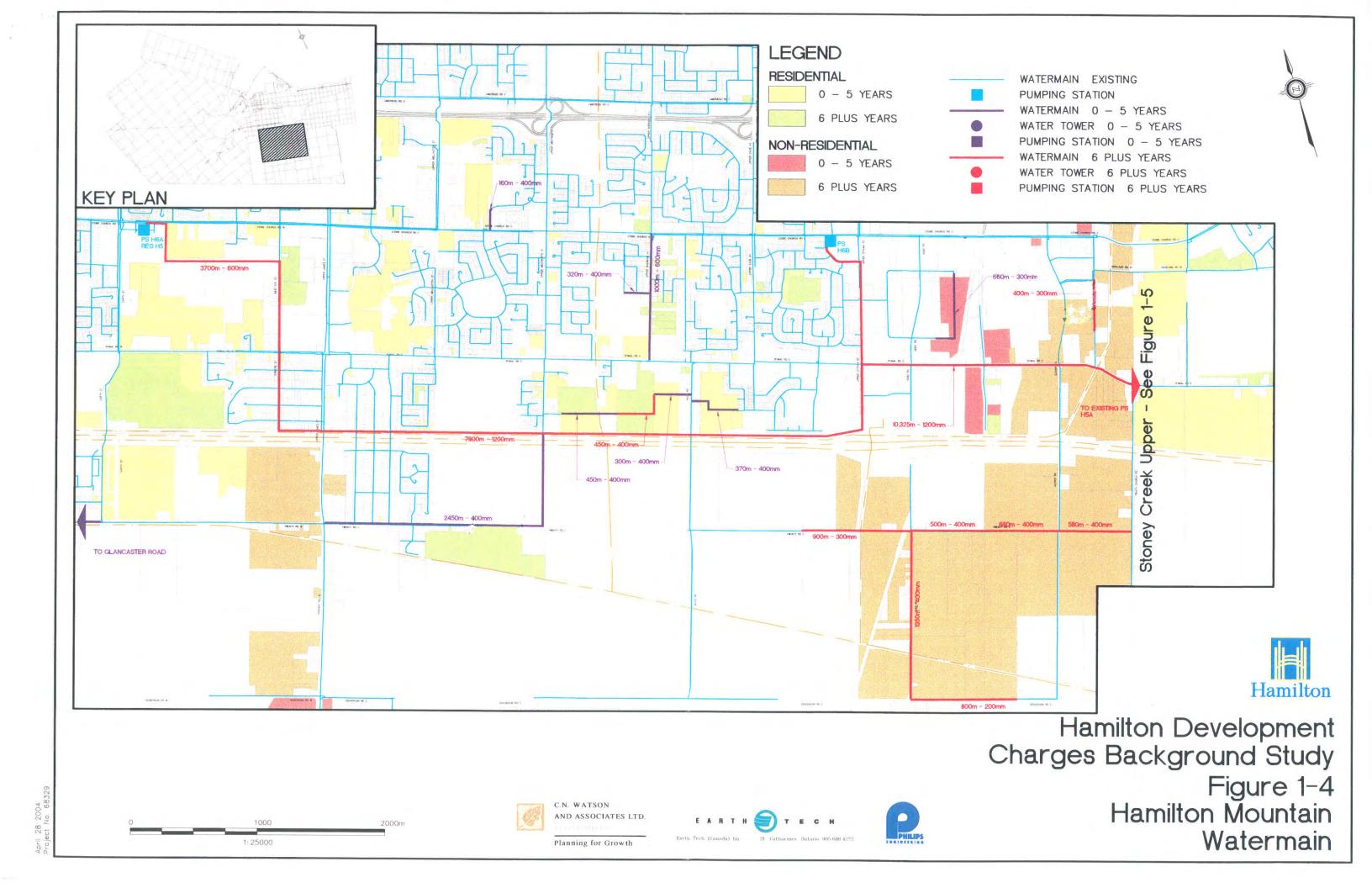


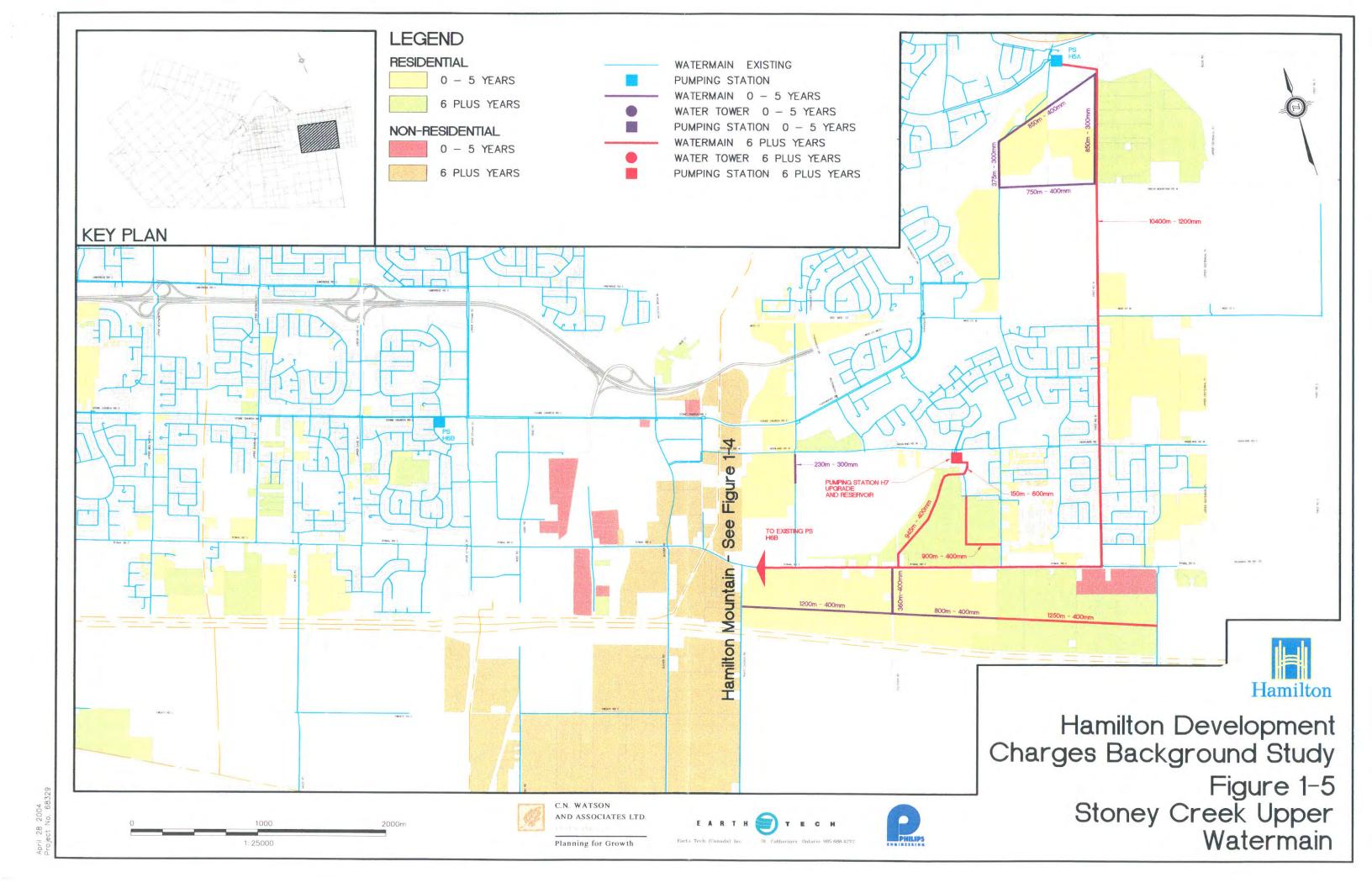
Hamilton Development Charges Background Study

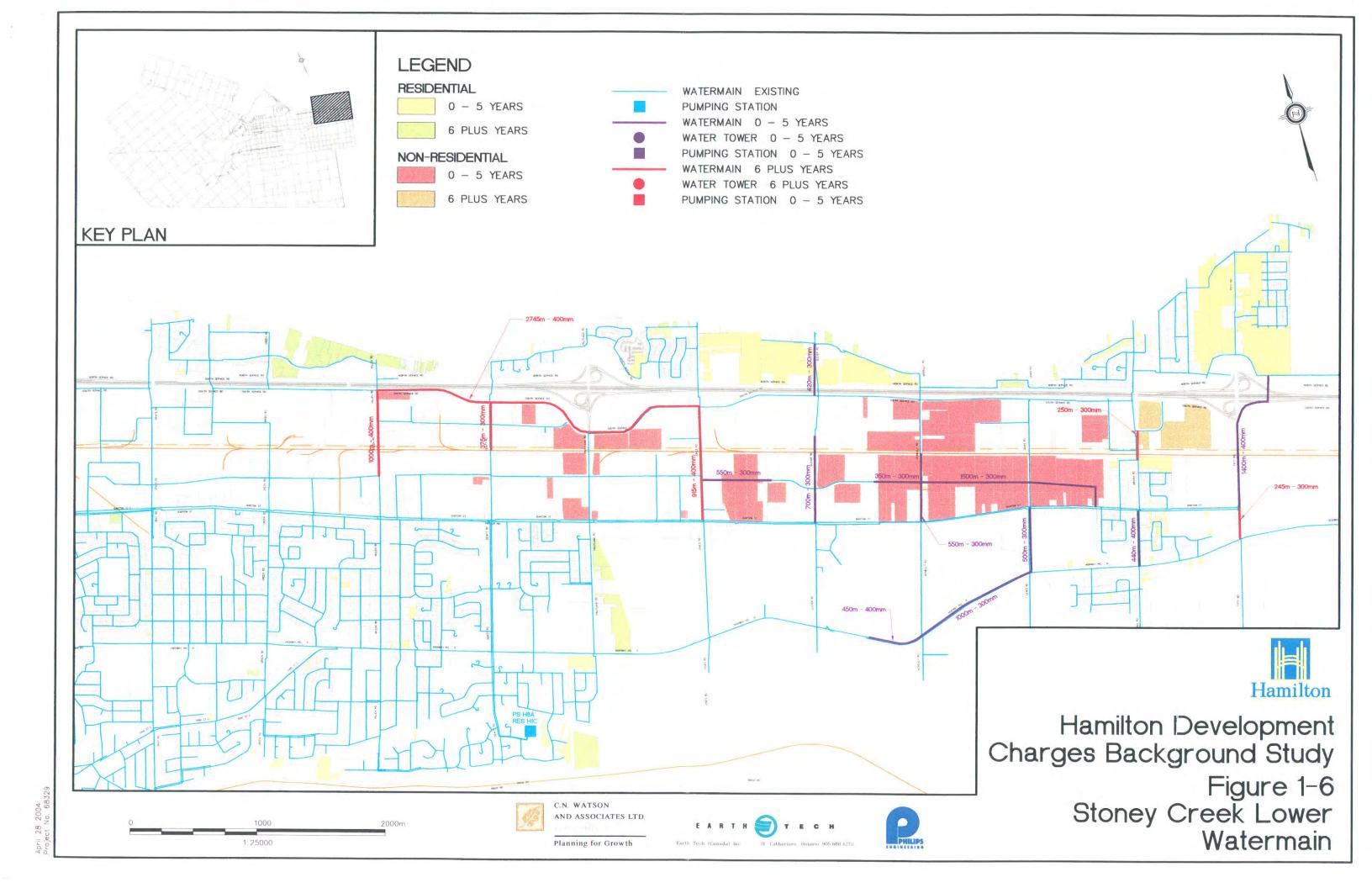
Figure 1-3
Binbrook / Mount Hope
Watermain

















Report III - Master Plan Class EA Report

Appendix A-3 (03)

Wastewater Capital Program
Master Plan Wastewater Servicing Projects



MASTER PLAN WASTEWATER SERVICING CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

	Code	Master	Туре	Ward	Project	Description	Size /	Quantity /		Class EA	Trigger	Comments
n Service		Plan Project No.					Total Capacity	Additional Capacity	Estimated Cost (Millions)	Schedule		
2007	MP	WW-1	Р	11, 12	HC018 - Twenty Road SPS Upgrades	Interim Upgrades and Ultimate Upgrades to Increase Capacity	590 L/s	590 L/s	\$2.6M	В	Growth in Hamilton Mountain (Mount Hope) and security/reliability of wastewater system operations	This project will address existing constraints as well as support localized growth in the SPS catchment including part of the Airport Lands.
2007	MP	WW-2	FM	11, 12	HC018 - Twenty Road SPS Forcemain	Twin Forcemain from Twenty Road SPS to MH-GD06A001	300 mm	1,400 m	\$1.4M	В	Growth in Hamilton Mountain (Mount Hope) and security/reliability of wastewater system operations	This project will address existing constraints as well as support localized growth in the SPS catchment including part of the Airport Lands.
2008	MP	WW-3	SPS	12	New HC008 - Harmony Hall SPS	Build a new SPS	100 L/s	100 L/s	\$5.5M	A	Growth in Ancaster and Airport Lands plus security/reliability of wastewater system operations	y In order to most efficiently service the Harmony Hall catchment, a single SPS at a more southerly location is proposed. This project is being addressed under local servicing upgrades.
2008	MP	WW-4	FM	12	New HC008 - Harmony Hall Forcemain	Install a new forcemain from new Harmony Hall SPS to MH- AM10A003	300 mm	1,000 m	\$0.7M	A	Growth in Ancaster and Airport Lands plus security/reliability of wastewater system operations	y The new forcemain will be connected to a more northerly section of sewer to avoid upgrading sections of sewer with lower capacities. This project is being addressed under local servicing upgrades.
2008	MP	WW-5	SPS	12	HC008 - Harmony Hall SPS Decommission	Decomission existing SPS		1 each	\$0.1M	В	Upgrades and relocation of HC008 Harmony Hall SPS	In order to most efficiently service the Harmony Hall catchment, a single SPS at a more southerly location is proposed. It is recommended that the new SPS replace the old SPS and that the old SPS be decommissioned.
2008	MP	WW-6	Р	11	HC016 - Winona SPS Upgrades	Install a third pump (88 L/s) at the existing HC016 SPS	176 L/s	1 each	\$0.2M	А	Growth in Stoney Creek	This project will provide additional pumping capacity as well as increase the flexibility and security of operations of the station with an additional pump
2008	MP	WW-7	FM	11	HC016 - Winona SPS Forcemain Twinning	Twin Existing Forcemain from Winona SPS to MH-SN02A003	300 mm	500 m	\$0.9M	Α	Growth in Stoney Creek	This project will provide additional pumping capacity as well as increase the flexibility and security of operations of the station with twinned forcemains
2008	MP	WW-8	WWTP	15	New Waterdown SPS	Build a new SPS	150 L/s	150 L/s	\$6.0M	В	Growth in Waterdown plus optimization of operations costs	This facility will be located at the existing WWTP site. All flows to the old WWTP will be pumped to the existing sewer on Dundas Street and conveyed to Hamilton
2008	MP	WW-9	FM	15	New Waterdown Forcemain	Build a new forcemain from new Waterdown SPS to MH-FL21A097	450 mm	1,600 m	\$1.8M	В	Growth in Waterdown plus optimization of operations costs	This project will convey flows to the existing sewer on Dundas Street
2008	MP	WW-10	WWTP	15	Waterdown WWTP	Decomission Plant		1 each	\$0.5M	В	Growth in Waterdown plus optimization of operations costs	It was determined that it is more cost efficient (long term life cycle costs) to decommission the plant than to expand and continue operating
2009	MP	WW-11	S	8	Ancaster-to-Fennell Trunk Sewer Twinning	Twin Existing Sewer from MH-HD15A010 to HD15A020	900 mm	400 m	\$0.8M	В	Growth in Hamilton Mountain, Ancaster and the Airport Lands	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
					Ancaster-to-Fennell Trunk Sewer Twinning	Twin Existing Sewer from MH-HD15A020 to HE15A090	1,050 mm	500 m	\$1.2M	В	Growth in Hamilton Mountain, Ancaster and the Airport Lands	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
					Ancaster-to-Fennell Trunk Sewer Twinning	Twin Existing Sewer from MH-HE15A090 to HF15E084	1,200 mm	1,500 m	\$3.9M	В	Growth in Hamilton Mountain, Ancaster and the Airport Lands	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
					Ancaster-to-Fennell Trunk Sewer Twinning	Twin Existing Sewer from MH-HF15E084 to HF14E011	1,350 mm	300 m	\$0.9M	В	Growth in Hamilton Mountain, Ancaster and the Airport Lands	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
2010	MP	WW-12	S	8	West 18th Street Sewer Twinning	Twin Existing Sewer from MH-HE18A020 to HF15A037	525 mm	2,000 m	\$3.3M	A	Growth in Hamilton Mountain	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
2010	MP	WW-13	S	2, 3, 4	Combined Sewer Overflow Control	Trunk CSO and collection system improvements in the East Harbour Area (Wellington Street to Dunn Avenue)			\$80.0M	С	Growth in Hamilton, Ancaster, Dundas, Waterdown, Stoney Creek plus providing wet weather flow control and meeting provincial treatment/capture targets	This project will involve a combination of system improvements in the East Harbour Area. The deisgn concept will be updated under a separate Phase 3 and 4 Class EA study.
2010	MP	WW-14	S	9, 11	New Centennial Trunk Sewer	New Gravity Trunk Sewer (open cut sections)	1,200 mm	7,000 m	\$24.5M	В	Growth in Hamilton Mountain, Airport Lands and Binbrook	This project will provide additional conveyance capacity to the Woodward Ave. WWTP for growth on the Mountain and will alleviate potential constraints on the Red Hill Creel Inteceptor
					New Centennial Trunk Sewer	New Gravity Trunk Sewer (sections requiring tunnelling)	1,200 mm	1,000 m	\$10.0M	В	Growth in Hamilton Mountain, Airport Lands and Binbrook	This project will provide additional conveyance capacity to the Woodward Ave. WWTP for growth on the Mountain and will alleviate potential constraints on the Red Hill Creel Inteceptor



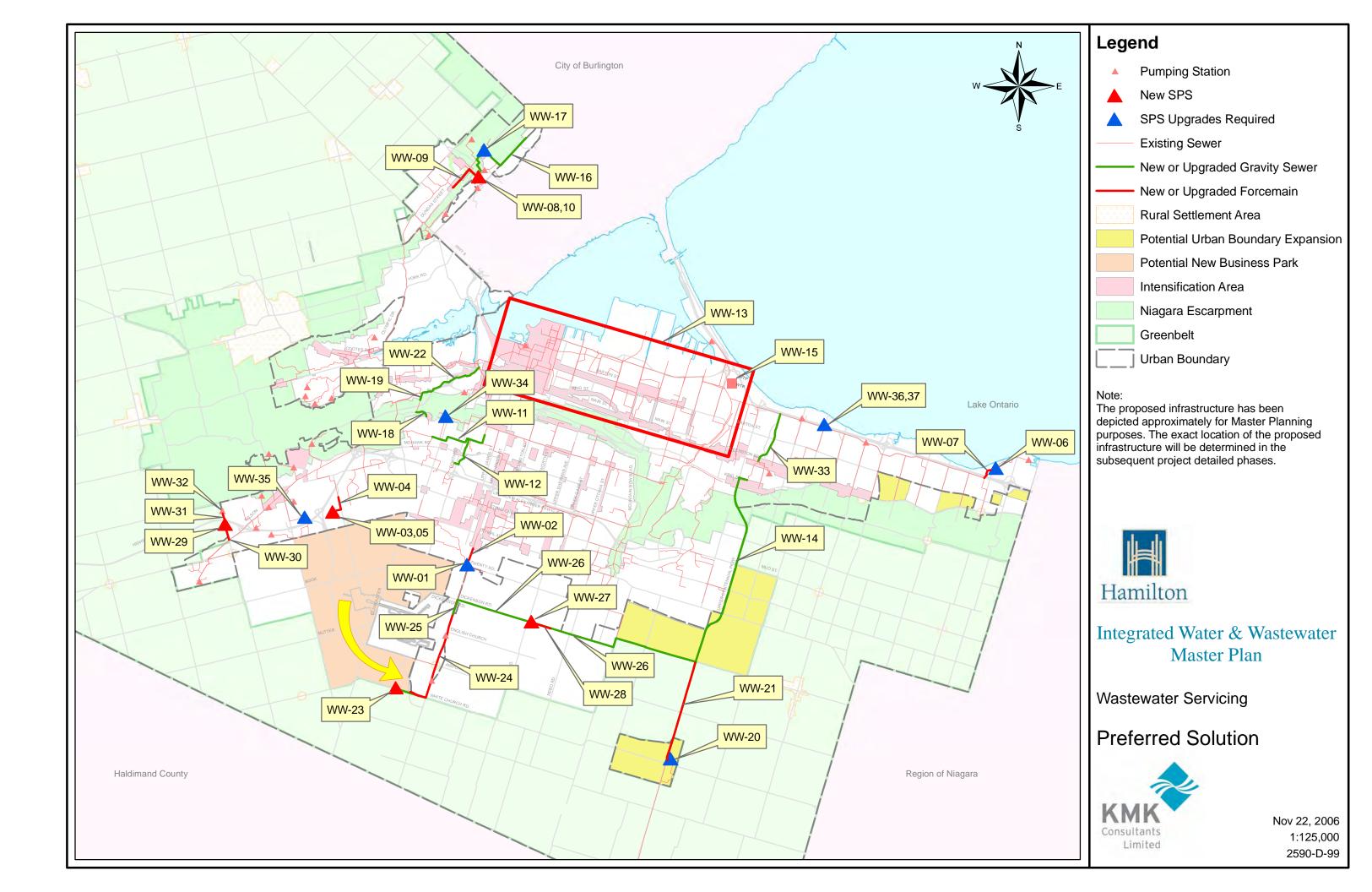
MASTER PLAN WASTEWATER SERVICING CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

Year Reg'd	Code	Master	Type	Ward Project	Description	Size /	Quantity /	Total	Class EA	Trigger	Comments
in Service		Plan Project No.	<i>,</i> ,		·	Total Capacity	Additional Capacity	Estimated Cost (Millions)			
2010	MP	WW-15	WWTP	4 Woodward Ave. WWTP Upgrades	WWTP Expansion			\$340.1M	С	Growth in Hamilton, Ancaster, Dundas, Waterdown, Stoney Creek plus providing wet weather flow control and meeting provincial treatment/capture targets	This project will be coordinated with the combination of system improvements in the East Harbour Area. The deisgn concept will be updated under a separate Phase 3 and 4 Class EA study.
2010	MP	WW-16	S	15 Mountain Brow Trunk Sewer	New Gravity Sewer to Service Waterdown South from the eastern limits to the First Street SPS	450 mm	3,000 m	\$2.3M	В	Growth in Waterdown South	This project's timing, location and capacity is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
				Mountain Brow Trunk Sewer	New Gravity Sewer to Service Waterdown South (Sections of Deep Sewers)	450 mm	500 m	\$1.6M	В	Growth in Waterdown South	This project's timing, location and capacity is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
2010	MP	WW-17	Р	15 DC014 - First Street SPS Upgrades	Install a third pump (200 L/s) and associated upgrades at the existing DC014 SPS	400 L/s	1 each	\$0.4M	В	Growth in Waterdown South and UpCounty	This project's timing and capacity is being coordinated through the Waterdown Class EA Study and OPA28 Secondary Plan process
2011	MP	WW-18	S	8 Scenic Drive Sewer Twinning	Sewer Twinning (MH-HD14A067 to MH HC13A060)	750 mm	500 m	\$1.3M	A	Growth in Hamilton Mountain	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
2012	MP	WW-19	S	1 Bowman Street Sewer Twinning	Sewer Twinning (MH HC13A010 to MH-HC12A057)	900 mm	500 m	\$1.0M	A	Growth in Hamilton Mountain	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
2011	MP	WW-20	Р	11 HC058 - Binbrook SPS Upgrades	Install a third pump (200 L/s) at the existing HC058 SPS	400 L/s	1 each	\$0.2M	В	Growth in Binbrook	This project will provide additional wastewater servicing capacity and security
2011	MP	WW-21	FM	11 Highway 56 Forcemain Twinning	Twin Existing 450mm Forcemain from HC058 to Golf Club Road	450 mm	4,200 m	\$3.8M	A	Growth in Binbrook	This project will provide additional wastewater servicing capacity and security. The forcemain will connect to the new Centennial Trunk Sewer. The existing forcemain will also be connected to the new Centennial Trunk Sewer.
2012	MP	WW-22	S	1 Hwy 403 Trunk Sewer Twinning	Twin Existing 900mm Sewer from Royal to Main-King (MH-HD12A002 to HE09A061)	900 mm	2,100 m	\$6.6M	В	Growth in Hamilton Mountain, Ancaster, the Airport Lands and west Hamilton	This sewer is located within the combined wastewater system and as such, receives high wet weather flows. Sewer twinning is required to meet peak wet weather flows.
2012	MP	WW-23	SPS	11 New Airport Lands SPS	New SPS in SE corner of the Airport Lands	500 L/s	500 L/s	\$7.5M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor
2012	MP	WW-24	FM	11 New Airport Lands Highway 6 Forcemain	New forcemain from new Airport Lands SPS to 1 km south of Dickenson Road	600 mm	4,700 m	\$4.7M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor. The alignment and length of forcemain will preclude other pumping station and forcemain upgrades on Hwy 6.
2012	MP	WW-25	S	11 New Highway 6 Trunk Sewer	New Gravity Sewer from 1 km south of Dickenson Road to Dickenson Road	900 mm	1,000 m	\$2.0M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor
2012	MP	WW-26	S	11 New Dickenson Road Trunk Sewer	New Gravity Sewer, all sections from Hwy 6 to Upper Centennial Pkwy	900 mm	9,600 m	\$15.4M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor. The infrastructure along Dickenson Road will preclude upgrades downstream and along Hwy 6. The infrastructure will also provide opportunity for servicing areas north of Dickenson Road.
2012	MP	WW-27	SPS	11 New Dickenson Road SPS	New SPS near Dickenson Road and Miles Road	500 L/s	500 L/s	\$7.5M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor. The infrastructure along Dickenson Road will preclude upgrades downstream and along Hwy 6. The infrastructure will also provide opportunity for servicing areas north of Dickenson Road.
2012	MP	WW-28	FM	11 New Dickenson Road Forcemain	New Forcemain from the new Dickenson Road SPS to approximately 400 m east of Miles Road	600 mm	400 m	\$0.5M	В	Growth in the Airport Lands	This project will provide sufficient capacity for the ultimate catchment area of the Airport Lands south of the existing utility corridor. The infrastructure along Dickenson Road will preclude upgrades downstream and along Hwy 6. The infrastructure will also provide opportunity for servicing areas north of Dickenson Road.



MASTER PLAN WASTEWATER SERVICING CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

Year Req'd in Service		Master Plan Project No.	Type	Ward Project	Description	Size / Total Capacity	Quantity / Additional Capacity	Total Estimated Cost (Millions)	Class EA Schedule	Trigger	Comments
2014	MP	WW-29	SPS	12 New HC053 - Shaver Road SPS	New SPS including additional pumping capacity	30 L/s	30 L/s	\$1.2M	В	Growth in Ancaster	The new SPS location will maximize the potential catchment area for servicing. This project will be updated as part of local servicing upgrades.
2014	MP	WW-30	FM	12 New HC053 - Shaver Road Forcemain	New Forcemain to Westview Drive	300 mm	500 m	\$0.4M	В	Growth in Ancaster	This project is required to support the new location for the SPS. This project will be updated as part of the local servicing upgrades
2014	MP	WW-31	S	12 New Shaver Road Sewer	New Gravity Sewer from old HC053 across Hwy 403 to new HC053 SPS	300 mm	500 m	\$1.2M	В	Growth in Ancaster	This project is required to support the new location for the SPS. This project will be updated as part of the local servicing upgrades
2014	MP	WW-32	SPS	12 HC053 - Shaver Road SPS Decommission	Decommission existing SPS		1 each	\$0.1M	В	Upgrades and relocation of HC053 Shaver Road SPS	In order to most efficiently service the Shaver Road catchment, a single SPS at a more southerly location is proposed. It is recommended that the new SPS replace the old SPS and that the old SPS be decommissioned.
2014	MP	WW-33	S	5,9 Battlefield Trunk Sewer Twinning	Twin Existing Sewer from MH-SD07A031 to HS08A002	900 mm	900 m	\$2.2M	Α	Growth in Hamilton Mountain and Airport Lands	This project will provide additional wastewater servicing capacity and security
				Battlefield Trunk Sewer Twinning	Twin Existing Sewer from MH-HS08A002 to HS07A044	1,200 mm	500 m	\$1.8M	А	Growth in Hamilton Mountain and Airport Lands	This project will provide additional wastewater servicing capacity and security
2016	MP	WW-34	Р	8 HC002 - Scenic Drive SPS Upgrades	Install a third pump (57 L/s) at the existing HC002 SPS	114 L/s	1 each	\$0.2M	В	Growth in Hamilton Mountain	This project will provide additional wastewater servicing capacity and security
2016	MP	WW-35	р	12 HC011 - Calvin Street SPS Upgrades	Install a third pump (59 L/s) at the existing HC011 SPS	118 L/s	1 each	\$0.2M	В	Growth in Ancaster and Airport Lands	This project will provide additional wastewater servicing capacity and security
2021	MP	WW-36	Р	10 HC056 - Green Road Upgrades	Install three new pumps (100 L/s each) at the existing HC056 SPS	200 L/s	3 each	\$0.8M	В	Growth in Stoney Creek	This project will provide additional wastewater servicing capacity and security
2021	MP	WW-37	FM	10 HC056 - Green Road Forcemain Twinning	Twin Existing Forcemain from Green Road SPS to SF03A011	300 mm	10 m	\$0.1M	В	Growth in Stoney Creek	This project will provide additional wastewater servicing capacity and security
				TOTAL COST				\$547.4M			









Report III - Master Plan Class EA Report

Appendix A-3 (04)

Wastewater Capital Program
Development Related Wastewater System Improvements



DEVELOPMENT-RELATED WASTEWATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

The following capital program was identified under the City of Hamilton Development Charge Background Study completed in 2004 by C. N. Watson in association with Earth Tech Canada and Philips Engineering. The capital program has been reviewed in coordination with the development of the Master Plan capital program. The total estimated costs have bee updated to reflect 2006 dollars.

W W		600 mm sewer from 700 m north of Parkside Dr. to Parkside Dr. 450 mm sewer from East to West	600 mm	700 m	Cost (Millions)	
W	/2 North Waterdown Area		600 mm	700 m		
W	/2 North Waterdown Area		600 mm	700 m		1
W		450 mm sewer from East to West		700 111	\$0.54M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
W	/3 North Waterdown Area	1	450 mm	900 m	\$0.59M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
		450 mm sewer from 700 m north of Parkside Dr. to Parkside Dr.	450 mm	700 m	\$0.46M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
W	4 South Waterdown	450 mm sewer from Evans Side Rd. to 480 m west of Evans Side Rd.	450 mm	480 m	\$0.32M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
	/5 South of Hwy. 5 E	525 mm sewer from 480 m west of Evans Side Rd. to 950 m east of Evans Side Rd.	525 mm	470 m	\$0.33M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
W	6 South of Hwy. 5 E	600 mm sewer from 950 m east of Evans Side Rd. to DC014 P.S.	600 mm	920 m	\$0.70M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
W	77 North of Mountain Brow Rd.	375 mm sewer from 465 m West of Kerns Rd to 1405 m west of Kerns Rd.				Covered by Master Plan Project No. WW-16
W	North of Mountain Brow Rd.	450 mm sewer from 930 m west of Kerns Rd. to 1405 m west of Kerns Rd.				Covered by Master Plan Project No. WW-16
W	North of Mountain Brow Rd.	525 mm sewer from 1405 m west of Kerns Rd. to proposed pumping station				Covered by Master Plan Project No. WW-16
W	10 Proposed P.S.	P.S. at western boundary of South Waterdown				Master Plan determined that South Waterdown can be most efficiently serviced by deep sewer and the SPS and forcemain are not required
W	11 Forcemain	300 mm forcemain from prop. P.S. to existing P.S.				Master Plan determined that South Waterdown can be most efficiently serviced by deep sewer and the SPS and forcemain are not required
W	12 DC014 Upgrade	Additional Standby Pump				Covered by Master Plan Project No. WW-17
W	13 DC014 Wet Well Upgrade	Additional Storage				Covered by Master Plan Project No. WW-17
W	14 Hwy. 6/Parkside Dr. Corner	375 mm sewer from 200 m south of Parkside Dr. to existing trunk north of Hwy. 5	375 mm	500 m	\$0.32M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
W	15 Hwy. 6/Parkside Dr. Corner	375 mm sewer from 300 m south of Parkside Dr. to existing trunk north of Hwy. 5	375 mm	300 m	\$0.19M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
А	1 Garner Rd	375 mm sewer from 480 m east of ex. PS to ex. PS	375 mm	450 m	\$0.29M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A:	2 Shaver Rd	250 mm sewer from 400 m north of Garner to Garner Rd.	250 mm	400 m	\$0.24M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	3 Garner Rd	375 mm sewer from Shaver Rd. to 100 m west of Shaver Rd.	375 mm	100 m	\$0.06M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	4 North of Garner Grove	600 mm sewer from North/West of development to south of Hwy. 403	600 mm	225 m	\$0.17M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	North of Garner Rd.	600 mm sewer from Garner Rd. to 225 m north of Garner Rd.	600 mm	225 m	\$0.17M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	6 Wilson St Replacement/Upgrade	750 mm sewer from 110 m south of Amberly to Radial Line	750 mm	2200 m	\$2.07M	Master Plan modeling analysis indicates that the Wilson Street Sewer Replacement is not required for additional capacity
A	7 Hamilton Dr.	250 mm sewer from Garner Rd. to 320 m North of Garner Rd.	250 mm	320 m	\$0.20M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	8 Ancaster Ind Park	375 mm sewer from 400 m east of Trinity Rd. to Cormorant Dr.	375 mm	800 m	\$0.51M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A	9 Garner Rd	250 mm sewer from Hamilton Dr. to 350 m west	250 mm	350 m	\$0.21M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A1	O Ancaster Ind Park	375 mm sewer from 300 m south of Garner Rd. to 100 m west of Shaver Rd.	375 mm	500 m	\$0.32M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
A1	11 HC053 PS	Replacement of HC053				Covered by Master Plan Project No. WW-29
A1	12 Shaver Rd	300 mm sewer from HC053 to 500 m south of Shaver Rd.				Covered by Master Plan Project No. WW-31
A1	3 Shaver Rd. Forcemain	150 mm FM from PS to Westview				Covered by Master Plan Project No. WW-30
A1	PS HC008 Upgrade	Additional Standby Pump				Covered by Master Plan Project No. WW-03
A1	15 PS-H26F				\$0.63M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
A1	6 Forcemain for PS H26F	300 mm FM from PS to Wilson 750 m east of Trinity	300 mm	1300 m	\$0.56M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
	W W W W A A A A A A A A A A A A A A A A	W8 North of Mountain Brow Rd. W9 North of Mountain Brow Rd. W10 Proposed P.S. W11 Forcemain W12 DC014 Upgrade W13 DC014 Wet Well Upgrade W14 Hwy. 6/Parkside Dr. Corner W15 Hwy. 6/Parkside Dr. Corner A1 Garner Rd A2 Shaver Rd A3 Garner Rd A4 North of Garner Grove A5 North of Garner Rd. A6 Wilson St Replacement/Upgrade A7 Hamilton Dr. A8 Ancaster Ind Park A9 Garner Rd A10 Ancaster Ind Park A11 HC053 PS	MS North of Mountain Brow Rd. 450 mm sewer from 930 m west of Kerns Rd. to 1405 m west of Kerns Rd. 525 mm sewer from 1405 m west of Kerns Rd. to 1705 mm sewer form 1405 mm west of Kerns Rd. 525 mm sewer from 1405 mm sewer from 1405 mm west of Kerns Rd. 525 mm sewer from 1405 mm s	Mile North of Mountain Brow Rd.	M8	North of Mountain Brow Rd.



DEVELOPMENT-RELATED WASTEWATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

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Year Req'd in	Code	DC	Project	Description	Size / Capacity	Quantity	Total Estimated	
Service		Project					Cost (Millions)	
		No.						
5 Years to UBBO	DC	A17	Trunk Sewer from HC053 to PS S-H26F	300 mm sewer from HC053 to Wilson Rd.	300 mm	600 m	\$0.37M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
5 Years to UBBO	DC	A18	Trunk Sewer from HC053 to PS S-H26F	450 mm sewer from Shaver to Garner	450 mm	1100 m	\$0.76M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
S Years to UBBO	DC	A19	Trunk Sewer from HC053 to PS S-H26F	600 mm sewer from Garner Rd. to Trinity Rd.	600 mm	1500 m	\$1.26M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
Years to UBBO	DC	A20	Trunk Sewer from HC053 to PS S-H26F	600 mm sewer from Wilson St. to PS H26F	600 mm	1300 m	\$1.09M	The needs for this project will be reviewed as part of Secondary Planning and/or local servicing requirements. Based on the limits of the Greenbelt, this project may be re-located or not required.
to 5 Years	DC	B1	Jackson Heights	250 mm sewer from 550 m north of Binbrook Rd to 260 m east of RR56	250 mm	260 m	\$0.16M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	B2	Jackson Heights	375 mm sewer from 550 m north of Binbrook Rd. to Binbrook Rd.	375 mm	550 m	\$0.35M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	В3	Elizabeth Gardens	750 mm sewer from 1650 m east of Fletcher Rd. to Regional Rd. 56	750 mm	420 m	\$0.39M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	B4	Elizabeth Gardens	375 mm sewer from 400 m west of Regional Rd. 56 to Regional Rd 56	375 mm	400 m	\$0.25M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	B5	Development South of Binbrook Rd.	450 mm sewer from Binbrook Rd to 450 m south	450 mm	750 m	\$0.49M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	В6	Development South of Binbrook Rd.	675 mm sewer from Binbrook Rd to 1650 m east of Fletcher Rd.	675 mm	400 m	\$0.34M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	В7	Regional Rd. 56	450 mm sewer from 400 m south of Binbrook Rd. to 900 m south of Binbrook Rd.	450 mm	500 m	\$0.45M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	B8	Development South of Binbrook Rd.	450 mm sewer from Binbrook Rd. to 450 m south	450 mm	750 m	\$0.49M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	В9	Development South of Binbrook Rd.	525 mm sewer from 420 m east of Fletcher Rd. to 1220 m East of Fletcher Rd.	525 mm	800 m	\$0.56M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	B10	PS HC058 upgrade	2 additional Pumps				Covered by Master Plan Project No. WW-20
6 Years to UBBO	DC	B11	Forcemain	450 mm FM from PS HC058 to Trinity Church Rd.				Covered by Master Plan Project No. WW-21
to 5 Years	DC	MH1	Southampton Estates	450 mm sewer from 600 m west of Homestead Dr. to Homestead Dr.	450 mm	680 m	\$0.45M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	MH2	Airport Ind Park	Trunk sewers			\$6.00M	The project requirements and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	MH3	PS HC059 and Forcemain	From PS HC059 to PS HC027				Covered by Master Plan Project No. WW-23, WW24
5 Years to UBBO	DC	MH4	Upgrades downstream of HC018					Covered by Master Plan Project No. WW-25, WW-26, WW-27, WW-28, WW-14, WW-33
Years to UBBO	DC	MH5	Upgrade to HC018					Covered by Master Plan Project No. WW-01
Years to UBBO	DC	MH6	Deferral 11	Twenty Rd to Dickinson			\$8.00M	The project needs, capacity and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	MH7	SPA 1 Non Res	Dickinson Rd. to Northern Airport Property Line			\$5.00M	The project needs, capacity and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	MH8	SPA 2 Non Res	Airport Rd., south to Hwy. 6			\$4.00M	The project needs, capacity and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
S Years to UBBO	DC	MH9	PS/FM Upgrades				\$0.70M	The project needs, capacity and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	HM1	Rymal Rd.	675 mm sewer from Trinity Church Rd. to 200 m west of Glover	675 mm	845 m	\$0.74M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	HM2	Miles/Sheral/Chappel Estates	300 mm sewer from 750 m west of Miles Rd. to Miles Rd	300 mm	750 m	\$0.47M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	HM3	Anchor Rd	375 mm sewer from Miles Rd. to 350 m east	375 mm	350 m	\$0.22M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC		West 5th St.	250 mm sewer from 250 m west of West 5th St. to West 5th St. 500 m north	250 mm	750 m	\$0.46M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	HM5	West 5th St.	375 mm sewer from south of Stone Church Rd. W to West of Upper James St.	375 mm	260 m	\$0.23M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	HM6	Commercial Dev between Dickenson Rd. and Twenty Rd.	450 mm sewer from Dickinson Rd. to 600 m north of Dickinson	450 mm	600 m	\$0.39M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the sout
Years to UBBO	DC	HM7	Commercial Dev between Dickenson Rd. and Twenty Rd.	525 mm sewer from 600 m north of Dickinson Rd. to 1120 m north of Dickinson	525 mm	520 m	\$0.37M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the sout



DEVELOPMENT-RELATED WASTEWATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

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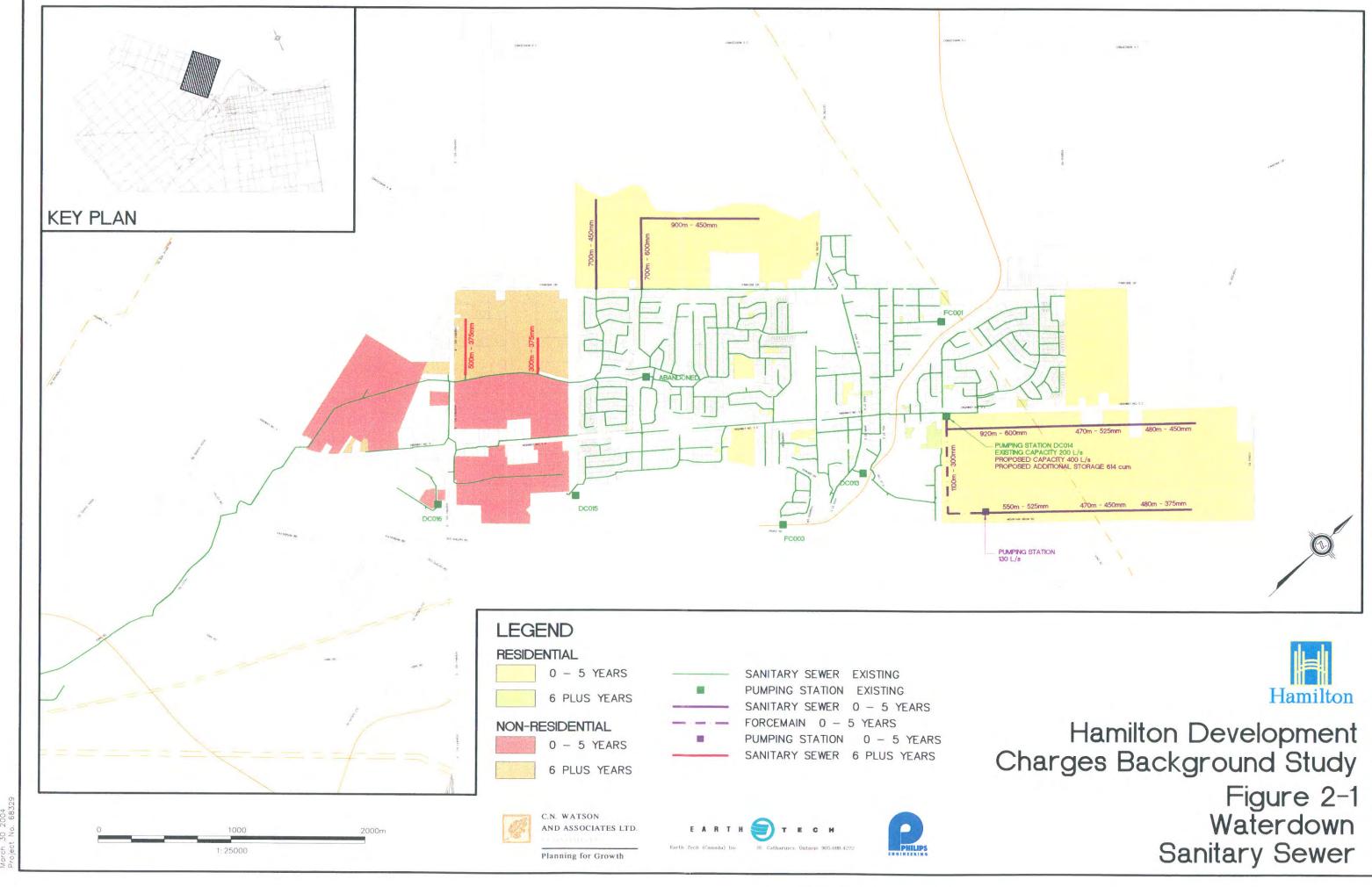
Year Req'd in	Code	DC Project	Description	Size / Capacity	Quantity	Total Estimated	Comments
Service		Project No.				Cost (Millions)	
Voors to LIPPO	DC	HM8 Glanbrook Ind Park	600 mm sewer from 1120 m north of Dickinson to Existing Trunk	600 mm	600 m	\$0.46M	The course alignment and timing will be reviewed as part of Secondary Planning and/or local convicing requirements. With the recommended
5 Years to UBBO	DC	nivio Gialibiook iliu raik	600 mm sewer nom 1120 m north of Dickinson to Existing Trunk	800 11111	600 111	\$0.46W	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the south.
5 Years to UBBO	DC	HM9 Nebo Rd.	375 mm sewer from 800 m south of Twenty Rd. to Twenty Rd.	375 mm	825 m	\$0.52M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the south.
Years to UBBO	DC	HM10 Nebo Rd.	375 mm sewer from 250 m north of Twenty Rd. to 425 m south of Rymal Rd.	375 mm	630 m	\$0.40M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	HM11 Upper Ottawa St.	375 mm sewer from 275 m north of Twenty Rd. to 350 m south of Rymal	375 mm	675 m	\$0.43M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	HM12 Twenty Rd.	375 mm sewer from 100 m west of Nebo Rd. to Nebo Rd.	375 mm	100 m	\$0.06M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the south.
5 Years to UBBO	DC	HM13 Twenty Rd.	450 mm sewer from Nebo Rd. to 650 m east of Nebo Rd.	450 mm	650 m	\$0.43M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. With the recommended Master Plan servicing strategy along Dickenson Road, the design of this project could be updated to coordinate with infrastructure to the south.
Years to UBBO	DC	HM14 Upper Sherman Ave.	250 mm sewer from 250 m north of Rymal Rd. to Acadia Dr.	250 mm	500 m	\$0.31M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
S Years to UBBO	DC	HM15 Properties east of Glover	375 mm sewer from 450 m south of Rymal Rd. to Rymal Rd.	375 mm	450 m	\$0.29M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
Years to UBBO	DC	HM16 Properties north of Rymal	375 mm sewer from Hydro corridor to 340 m west of Hydro corridor	375 mm	340 m	\$0.22M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
S Years to UBBO	DC	HM17 Pritchard Rd.	375 mm sewer from 140 m north of Rymal Rd. to 400 m north of Rymal Rd	375 mm	260 m	\$0.16M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
S Years to UBBO	DC	HM18 PS HC018 Upgrade	2 Pumps				Covered by Master Plan Project No. WW-01
S Years to UBBO	DC	HM19 PS HC018 FM Twinning					Covered by Master Plan Project No. WW-02
5 Years to UBBO	DC	HM20 Hwy. 6 north of HC018	250 mm sewer from HC018 to 100 m north	250 mm	300 m	\$0.18M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	SCU1 Properties north of Green Mountain Rd.	300 mm sewer from 400 m west of First Rd. to First Rd.	300 mm	750 m	\$0.47M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. This project will need to be coordinated with the recommended Master Plan servicing strategy to service the Nash neighbourhood with the new Centennial Trunk sewer.
to 5 Years	DC	SCU2 Drop Structure	450 mm sewer from Escarpment to Greenhill Ave	450 mm	150 m	\$1.44M	The recommended Master Plan servicing strategy is to service the Nash neighbourhood with the new Centennial Trunk sewer. Local servicing requirements such as a SPS in lieu of the drop structure may be required to support this strategy.
to 5 Years	DC	SCU3 Properties south of Mud St.	375 mm sewer from 200 m west of Upper Centennial Pkwy to First Rd.	375 mm	775 m	\$0.49M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	SCU4 Properties east of Fletcher Rd.	375 mm sewer from 150 m east of Fletcher Rd. to Fletcher Rd.	375 mm	300 m	\$0.19M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	SCU5 Rymal Rd.	450 mm sewer from 320 m west of First Rd. to 2nd Rd.			\$1.50M	This strategy was confirmed through the ROPA9 Class EA. However, the recommended long term solution from the Master Plan provides for the eastern part of ROPA9 to be serviced by gravity by the Centennial Trunk Sewer.
to 5 Years	DC	SCU6 Rymal Rd.	600 mm sewer from 2nd Rd. to 820 m west of 2nd Rd.			\$2.00M	This strategy was confirmed through the ROPA9 Class EA. However, the recommended long term solution from the Master Plan provides for the eastern part of ROPA9 to be serviced by gravity by the Centennial Trunk Sewer.
to 5 Years	DC	SCU7 Rymal Rd.	675 mm sewer from 820 m west of 2nd Rd. to Trinity Church Rd.			\$2.30M	This strategy was confirmed through the ROPA9 Class EA. However, the recommended long term solution from the Master Plan provides for the eastern part of ROPA9 to be serviced by gravity by the Centennial Trunk Sewer.
to 5 Years	DC	SCU8 Upper Mount Albion	250 mm sewer from 310 m south of Mud St. to 75 m north of Mud St.	250 mm	385 m	\$0.46M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	SCU9 Highland Rd. W	375 mm sewer from Upper Mount Albion to east of Upper Mount Albion	375 mm	390 m	\$0.25M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
to 5 Years	DC	SCU10 Candlewood Dr.	300 mm sewer	300 mm	160 m	\$0.10M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	SCU11 Properties east of First Rd.	375 mm sewer from 150 m north of Green Mountain to First Rd.	375 mm	1075 m	\$0.68M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. This project will need to be coordinated with the recommended Master Plan servicing strategy to service the Nash neighbourhood with the new Centennial Trunk sewer.
6 Years to UBBO	DC	SCU12 Properties east of Fletcher Rd.	375 mm sewer from 500 m north of Rymal Rd. to 150 m east of Fletcher	375 mm	415 m	\$0.26M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
5 Years to UBBO	DC	SCU13 Proposed P.S.				\$1.00M	This strategy was confirmed through the ROPA9 Class EA. However, the recommended long term solution from the Master Plan provides for the eastern part of ROPA9 to be serviced by gravity by the Centennial Trunk Sewer.
5 Years to UBBO	DC	SCU14 Forcemain	250 mm FM from 350 m south of Rymal to 320 m west of First Rd			\$1.10M	This strategy was confirmed through the ROPA9 Class EA. However, the recommended long term solution from the Master Plan provides for the eastern part of ROPA9 to be serviced by gravity by the Centennial Trunk Sewer.

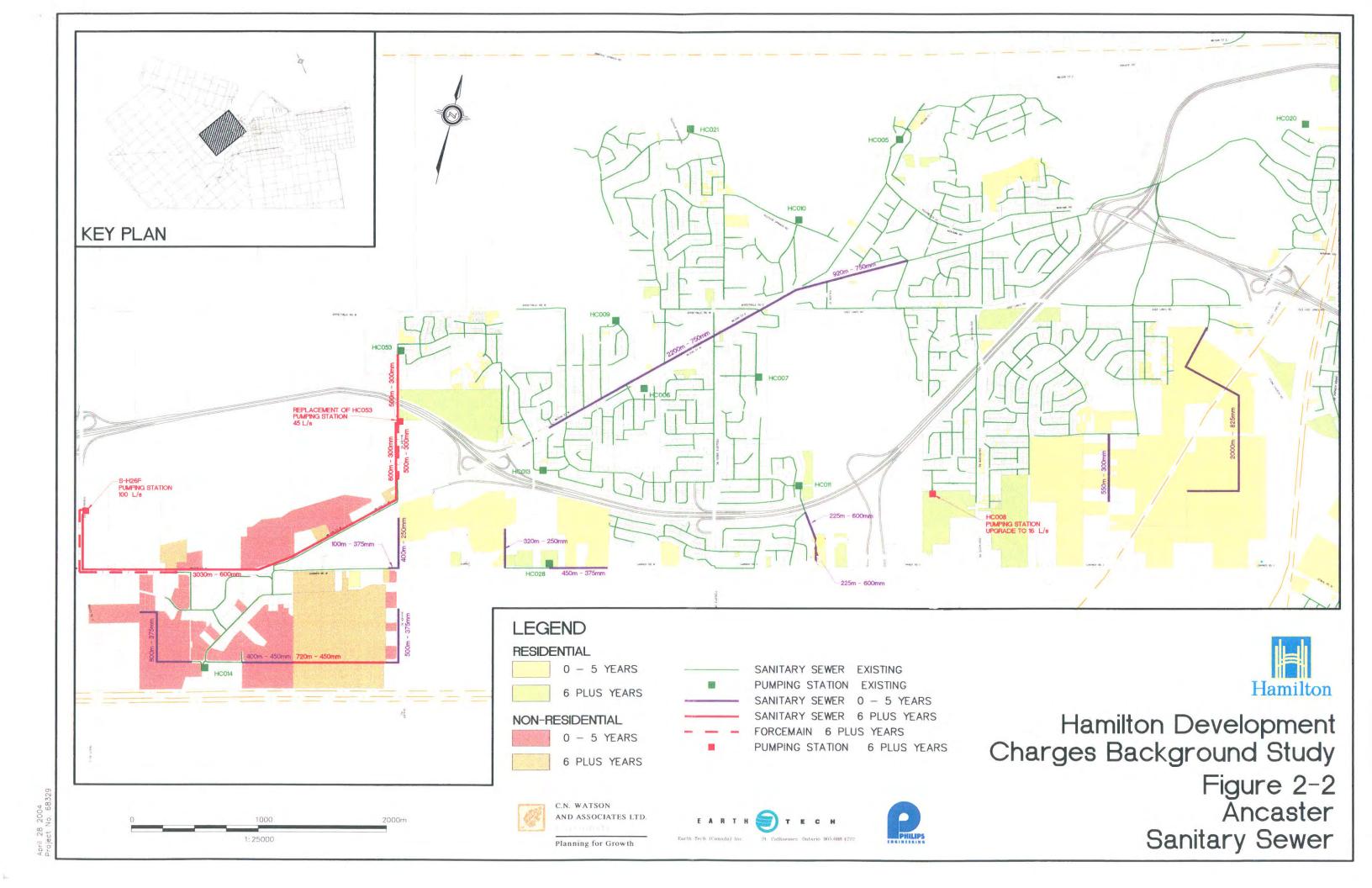


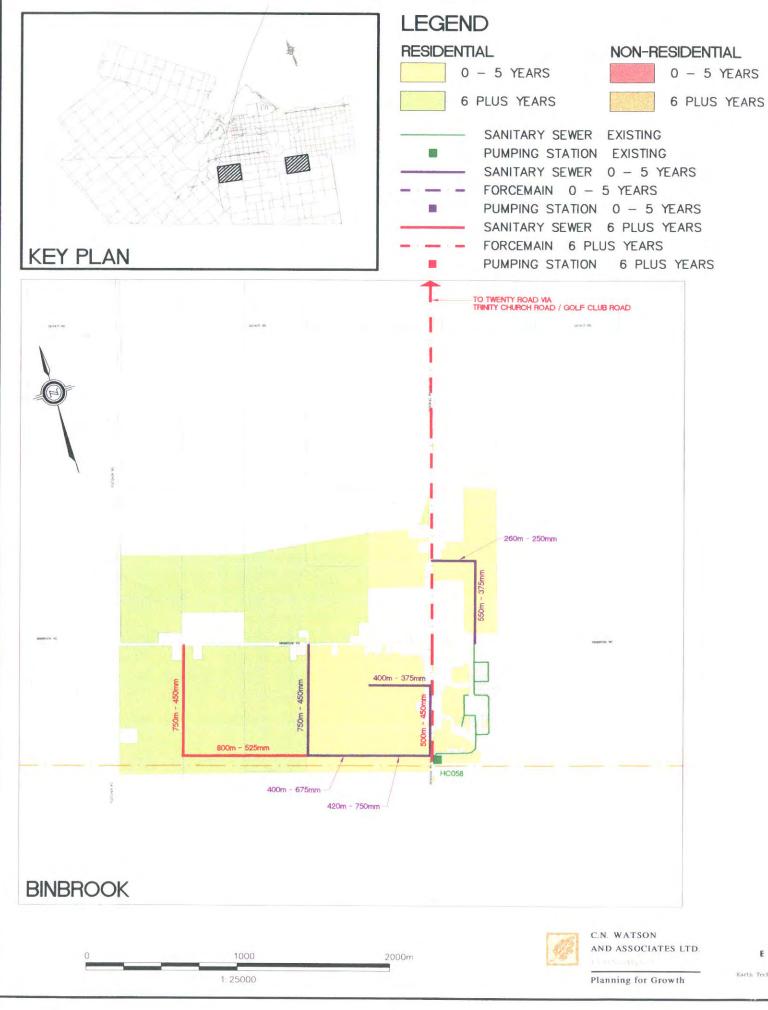
DEVELOPMENT-RELATED WASTEWATER SYSTEM IMPROVEMENTS CAPITAL PROGRAM - 22 NOVEMBER 2006 2006 DOLLARS

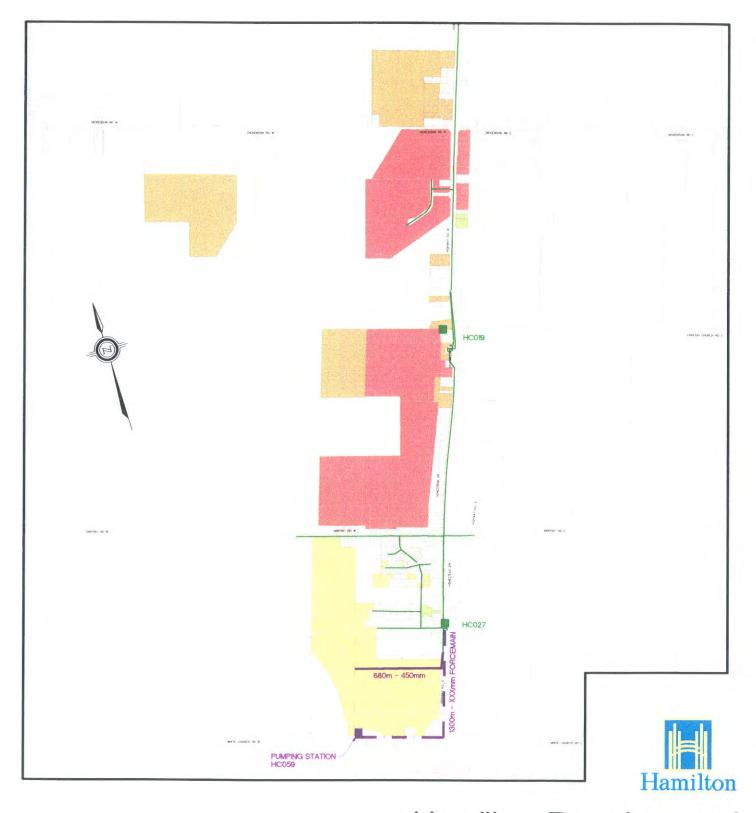
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Year Req'd in Service	Code	DC Project No.	Description	Size / Capacity	Quantity	Total Estimated Cost (Millions)	
0 to 5 Years	DC	SCL1 Falcon Rd	250 mm sewer from Fifty Rd. to Existing Gravity Sewer	250 mm	220 m	\$0.13M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL2 Arvin Ave Extension	375 mm sewer from 250 m east of McNeilly Rd to McNeilly Rd.	375 mm	250 m	\$0.16M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL3 Arvin Ave Extension	375 mm sewer from 300 m west of Lewis Rd. to Lewis Rd.	375 mm	300 m	\$0.19M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL4 Arvin Ave Extension	375 mm sewer from 250 m west of McNeilly Rd. to McNeilly Rd.	375 mm	250 m	\$0.16M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL5 Lands East of Jones Rd	375 mm sewer from Jones Rd. to 250 m east of Jones Rd.	375 mm	250 m	\$0.16M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL6 Lands East of Lewis Rd	375 mm sewer from Lewis Rd. to 300 m east of Lewis Rd.	375 mm	490 m	\$0.31M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	SCL7 Extension to Bridge Port Subdivision	300 mm sewer from Ex. Off Jones Rd. to Bridge Port Subdivision	300 mm	230 m	\$0.14M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL8 Lakeside Dr.	250 mm sewer from 300 m west of Jones Rd. to Jones Rd.	250 mm	300 m	\$0.18M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL9 Commercial Dev	375 mm sewer	375 mm	330 m	\$0.15M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL10 South Service Rd	600 mm sewer from Fifty Rd. to 250 m west of Winona	600 mm	1100 m	\$0.84M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL11 North Service Rd.	250 mm sewer from Millen Rd to Dewitt Rd.	250 mm	900 m	\$0.55M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL12 Forcemain - South Service Rd.	200 mm FM from PS to Fifty Rd.	200 mm	450 m	\$0.27M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
6 Years to UBBO	DC	SCL13 PS at South Service Rd. and Fifty Rd.		235 L/s		\$0.53M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements
0 to 5 Years	DC	New Longwood Road	1200 mm sewer for the McMaster Innovation Park to the Hwy 403 trunk sewer	1200 mm	500 m	\$0.53M	The sewer alignment and timing will be reviewed as part of Secondary Planning and/or local servicing requirements. This project could be coordinated with Master Plan Project No. WW-22.
		Total Cost				\$62.88M	









Hamilton Development Charges Background Study Figure 2-3

Figure 2-3
Binbrook / Mount Hope
Sanitary Sewer





